123 Washington Street Columbus, Indiana 47201 Phone: (812) 376-2550 Fax: (812) 376-2643





STAFF REPORT

CITY OF COLUMBUS PLAN COMMISSION (December 9, 2015 Meeting)

Docket No. / Project Title: MP-15-08 (Westwood 5th Replat)

Staff: Allie Keen

Applicant: Mark Pratt
Property Size: 6.73 Acres

Current Zoning: PUD (Westwood Planned Unit Development)

Location: Southeast corner of the intersection of State Road 46 and Westwood

Boulevard, in the City of Columbus.

Background Summary:

The applicant has indicated that the proposed subdivision is for the purpose of creating one new lot consisting of 2 acres. The proposed new lot fronts State Road 46 which is classified as a principle arterial street by the Columbus Thoroughfare Plan. The Subdivision Control Ordinance requires the installation of any required sidewalks with the minor subdivision process. The sidewalk requirement for arterial streets is determined by the Plan Commission.

Also, the applicants are requesting a modification from Subdivision Control Ordinance Section 16.24.010(B)(1) to delay the requirement to install sidewalks along Westwood Boulevard, which is classified as a local street, until the property develops.

Key Issue Summary:

The following key issue(s) should be resolved through the consideration of this application:

- 1. Does the proposed modification to not install the required sidewalks along the Westwood Boulevard frontage meet the modification decision criteria?
- 2. Should sidewalks be required along the State Road 46 frontage as a part of the subdivision?
- 3. Should the existing "No Access" easement be removed along the Westwood Boulevard frontage of the new lot?

Preliminary Staff Recommendation:

Approval of the subdivision, subject to the "No Access" remaining as previously shown and without the requirement to install sidewalks along the State Road 46 frontage due to the topography and existing conditions along this frontage that make a sidewalk not feasible at this location.

Staff also recommends approval of the modification request to delay the sidewalk requirement along Westwood Boulevard until the lot is developed. This approval of the subdivision should be subject to the following condition: A note shall be added to the plat stating the following: A sidewalk meeting the applicable width, construction, and location standards of the Subdivision Control Ordinance shall be installed along the Westwood Boulevard frontage of Lot 6 at the time it is developed. Sidewalk construction along the Westwood Boulevard frontage of Block A-B shall be determined by the applicable regulations at the time of its platting and/or development.

Plan Commission Options:

In reviewing a request for <u>minor subdivision approval</u>, which includes a request for modification(s) the Plan Commission may (1) approve the modification(s) and the subdivision, (2) deny the modification(s) and the subdivision, or (3) continue the review to the next Plan Commission meeting. If more than one modification is requested, the Plan Commission may approve some, all, or none of the requests. The Plan Commission may attach conditions to the approval of the modification(s); however the conditions should be directly related to the circumstances of the modification requested. (Per Subdivision Control Ordinance Section 16.40.050)

Modification Decision Criteria:

Indiana law and Columbus Subdivision Control Ordinance Section 16.40.050 require that the Plan Commission approve all subdivisions which meet the applicable requirements. The Columbus Plan Commission has delegated the approval authority for Minor Subdivisions to the Plat Committee. This application is being considered by the Plan Commission because the Plat Committee found at least one aspect of the proposal to be inconsistent with the Subdivision Control Ordinance. Section 16.32.020 of the Subdivision Control Ordinance indicates that the Plan Commission may grant a modification of the requirements of the Ordinance upon the determination that each of the following is true:

- 1. The modification will not be detrimental to the public, health, safety, and general welfare.
- 2. Adjacent property will not be adversely affected.
- 3. The modification is justified because of exceptional topographic or other physical conditions unique to the property involved, as opposed to mere inconvenience or financial disadvantage.
- 4. The modification is consistent with the intent of the Zoning Ordinance, other applicable ordinances, and the Comprehensive Plan.
- 5. The modification will not conflict with the requirements of the Zoning Ordinance including but not limited to lot area, lot frontage, lot width, and setback(s).

Outstanding Technical Comments:

The following outstanding technical comments must be addressed by the applicant:

- 1. In the property description, "Block A1" should be labeled as "Block A-A" to be consistent with the previous plat.
- 2. In the site location map, please outline or label the entire Westwood subdivision.
- 3. The Chairman for Plan Commission is "Roger Lang" and the secretary is "David Fisher."
- There is nothing that addresses who utilizes/maintains the detention easement on the plat or in the owner's certificate. This should be addressed on both this minor subdivision plat and the administrative plat (Case# AD-15-27).

Current Property Information (entire subdivision site):				
Land Use:	Vacant/Undeveloped			
Site Features:	There are no significant site features at this location.			
Flood Hazards:	There are no flood hazards at this location.			
Special Circumstances: (Airport Hazard Area, Wellfield Protection Area, etc.)	There are no special circumstances at this location.			

Vehicle Access:	This property can only gain access from Westwood Boulevard (Local, Commercial, Suburban). There is no access platted along the State Road 46 frontage and a portion of the Westwood Boulevard frontage.
	46 frontage and a portion of the viestwood Boulevard frontage.

Surroundi	Surrounding Zoning and Land Use (entire subdivision site):				
	Zoning:	Land Use:			
North:	CR (Commercial: Regional)	Commercial (Westhill Shopping Center)			
South:	PUD (Westwood Planned Unit Development)	Multi-Family (Westwood Pines Apartments)			
East:	PUD (Woodcrest Planned Unit Development)	Two-Family Residential Office			
West:	PUD (Westwood Planned Unit Development)	Retail (CVS Pharmacy) Office			

Interdepartmental R	eview:
City Engineering:	The "No Access" easement should remain as previously shown. If a drive is allowed in that area it will result in vehicles making turns across a thru lane and turn lane to access the property. Also, this drive may be blocked by the queue from the signalized intersection.
City Utilities:	No Comments.
Parks Department:	No Comments.
INDOT:	It appears this location has a guardrail the length of the frontage as well as turn lanes for the intersection in front of the property. With these two things in consideration we would not allow access onto State Road 46, so I am not sure why the no access should be removed.

History of this Location:

The relevant history of this property includes the following:

- 1. On October 5, 1994, the Columbus Plan Commission approved the Westwood Major Subdivision Preliminary Plat (PP-94-9). The preliminary plat consisted of 2 public streets and 3 blocks consisting of 39.27 acres. The three blocks were intended to be broken up into separate building lots in order to develop the property.
- 2. On August 27, 1997, a final plat (FP-97-9) for Westwood was approved and consisted of 4 lots and 3 blocks intended for future development. The subject property is Block A of this final plat.
- 3. On February 6, 2001, a minor subdivision (MP-01-02) was approved that created Lot 1B, where CVS is currently located.
- 4. On February 15, 2009, a minor subdivision (MP-08-01) was approved that created Lot 5A, where Residence Inn is located.

History of this Application:

The relevant history of this application includes the following: This application was reviewed by the Columbus Plat Committee at its November 19, 2015 meeting. The following comments are unresolved:

- 1. Plan Commission needs to make the determination whether sidewalks along State Road 46, which is a principle arterial street, should be required to be installed as a part of the subdivision request.
- 2. The applicants needed to either install the required sidewalks along the Westwood Boulevard frontage or request a modification to waive the sidewalk requirement.
- 3. The Plan Commission needs to determine whether the existing "No Access" easement should remain as it is or be removed along Westwood Boulevard.

Planning Consideration(s):

The following general site considerations, planning concepts, and other facts should be considered in the review of this application:

- The applicant is proposing to create 1 new lot within the Westwood Major Subdivision that is 2 acres in size and located at the southeast corner of the intersection of State Road 46 and Westwood Boulevard.
- 2. The proposed lot has frontage along Westwood Boulevard which is identified as a local street. The Subdivision Control Ordinance requires a 5 foot wide sidewalk to be installed along this frontage as a part of the minor subdivision process, per Section 16.24.010(B)(1). The applicants are requesting to delay this requirement as a part of the subdivision process. They would instead prefer to install the sidewalk when the property is developed. Per Zoning Ordinance Section 7.3(Part 2)(A), all development in commercial zoning districts shall provide public sidewalks in all adjoining street and road rights-of-way.
- 3. Subdivision Control Ordinance Section 16.24.010(B)(1) also states that the sidewalk requirement along arterial streets shall be determined by the Plan Commission. The proposed lot also has frontage on State Road 46, which is identified as a principle arterial. Currently, there are no sidewalks along either the north or south sides of State Road 46 in this area until the Goeller Boulevard intersection where the People Trail begins on the north side of the street. Although a sidewalk connection between the Westwood Development and this People Trail is desirable along State Road 46, the existing topography would make it difficult to install a sidewalk.
- 4. Currently there is a platted "No Access" easement along the entire State Road 46 frontage and along approximately 105 feet of the Westwood Boulevard frontage of the subject property. This "No Access" notation was shown on both the Preliminary and Final Plats for Westwood. With the proposed plat, this "No Access" easement would limit the access to the new lot to approximately 84 feet along Westwood Boulevard. The applicants are proposing to remove the no access easement along Westwood Boulevard with the proposed plat in order to provide more flexibility when developing the new lot. It is at the Plan Commission's discretion as to whether or not the "No Access" easement can be removed because it was a part of the approved preliminary plat for Westwood. The City Engineering Department has recommended that the no access easement remain as it is currently platted for the purpose of preventing an access point close to the State Road 46 intersection. They have stating that if a driveway was permitted at this location it would result in vehicles accessing the property by crossing a thru lane and turn lane. Additionally Engineering stated the drive may become blocked by a queue from the signalized intersection.
- 5. In February of 2001, a minor subdivision was approved for the creation of Lot 1B, which is where CVS is currently located. On that final plat the original "No Access" easement was removed from the Westwood Boulevard frontage. The easement now only extends approximately 13.67 feet south on Westwood Boulevard. The Westwood PUD document does not specify a required separation distance for drives; therefore it is at the Plan Commission's discretion with the Final PUD approval for each lot. For comparison, the Zoning Ordinance requires a minimum of 100 feet of separation between two access points along a local street, per Section 7.3(Part 1)(C)(3)(c)(iii). CVS has two access drives from Westwood Boulevard. The northern drive is separated approximately 144.5 feet from State Road 46 and approximately 139 feet from the southern drive. When the proposed lot is developed, it is likely their access point/points will align with the existing CVS property.

Subdivision Control Ordinance Consideration(s):

The following provisions of the Subdivision Control Ordinance apply to the key issues of this application: Section 16.24.010(B)(1) Improvement of Existing Streets: Minor subdivisions shall be required to (1) dedicate right-of-way and (2) install any required sidewalks along the frontage of all new lots, with each consistent with the Thoroughfare Plan classification of adjacent streets and the specification of the City Engineer for arterial streets. The sidewalk requirement shall be determined by the Plan Commission for arterial streets.



NOVEMBER 24, 2015

COLUMBUS / BARTHOLOMEW PLANNING DEPARTMENT JEFFREY R. BERGMAN, AICP PLANNING DIRECTOR 123 WASHINGTON ST. SUITE 8 COLUMBUS, INDIANA 47201

RE: REQUEST FOR MODIFICATION WESTWOOD 5th REPLAT (MP-15-08)

PLEASE ACCEPT THIS LETTER AS A FORMAL REQUEST TO SEEK MODIFICATION, FROM THE CITY OF COLUMBUS SUBDIVISION CONTROL ORDINANCE, OF THE CITY OF COLUMBUS PLAN COMMISSION.

WE ARE REQUESTING PARTIAL RELIEF FROM THE REQUIREMENT OF SIDEWALK CONSTRUCTION AS DESCRIBED IN SECTION 16.24.010(B)(1); OF THE CITY OF COLUMBUS SUBDIVISION CONTROL ORDINANCE OF CURRENT ADOPTION. IT IS REQUESTED THAT THE SIDEWALK BE DESIGNED DURING THE SITE PLAN PROCESS AND CONSTRUCTED WHEN THE SITE IS DEVELOPED.

IF YOU HAVE ANY QUESTIONS OR NEED ADDITIONAL INFORMATION, PLEASE GIVE ME A CALL.

THANK YOU,

RIK Z. SANDERS
SENIOR DIRECTOR, SURVEYS

ERG JOB #15383

Westwood 5th Replat OWNERS' CERTIFICATE I THE UNDERSIGNED, MARK A. PRATT, PRESIDENT OF BREEDEN INVESTMENT GROUP, INC. OWNER OF THE REAL ESTATE SHOWN AND DESCRIBED HEREIN, DO HEREBY A PART OF THE NORTHEAST QUARTER OF SECTION 28, TOWNSHIP 9 NORTH, RANGE 5 EAST CERTIFY THAT WE HAVE LAID OFF, PLATTED AND SUBDIVIDED, AND DO HEREBY LAY OFF, PLAT AND SUBDIVIDE, SAID REAL ESTATE IN ACCORDANCE WITH THE WITHIN PLAT. COLUMBUS TOWNSHIP, BARTHOLOMEW COUNTY, INDIANA STATE ROAD #46 N52:17:52:W THIS SUBDIVISION SHALL BE KNOWN AND DESIGNATED AS "WESTWOOD 5th REPLAT" SHEET 1 OF 1 (PRINCIPLE ARTERIAL, COMMERCIAL, SUBURBAN) CONSISTING OF ONE LOT IDENTIFIED HEREON AS "LOT 6" AND ONE BLOCK IDENTIFIED HEREON AS "BLOCK A-B" AND CONTAINING IN ALL 6.73 ACRES. NORT S87'42'12"W 284.80' CLEAR TITLE TO THE LAND CONTAINED IN THIS SUBDIVISION IS GUARANTEED S87°32'26"W ACREAGE TABLE THE SETBACK LINES SHALL BE DETERMINED BY THE REGULATIONS OF THE WESTWOOD PLANNED UNIT DEVELOPMENT. LOT 6 2.00 Ac. THERE ARE STRIPS OF GROUND SHOWN ON THIS PLAT AND MARKED EASEMENT, RESERVED FOR THE USE OF PUBLIC UTILITIES AND SUBJECT TO THE PARAMOUNT RIGHT OF THE UTILITY OR CITY TO INSTALL, REPAIR, MAINTAIN OR REPLACE ITS Ϋ́ BLOCK A-B 4.73 Ac. -NO ACCESS TOTAL. 6.73 Ac. S00'13'27"W 54.54' "MOODCREST A REPLAT OF REMOVE NO ACCESS ALL DRAINAGE EASEMENTS IDENTIFIED ON THE PLAT ARE SPECIFICALLY AUTHORIZED TO BE USED FOR DRAINAGE PURPOSES. ALL GRADES SHALL BE MAINTAINED AS CONSTRUCTED. ADDITIONAL CUT AND FILL WORK WITHIN DRAINAGE EASEMENT AREAS LOTS 5,6,37" PB "K", F64 LOT 6 IS PROHIBITED UNLESS AUTHORIZED BY THE ENGINEERING DEPARTMENT. S13'59'16"W 50.45' SCALE: 1" == 100' BOULEVARD BLOCK A-A IN "WESTWOOD 4th REPLAT" IS HEREBY VACATED BY THIS PLAT. 2.00 ACRES LOT 6 AND BLOCK A-B ARE SUBJECT TO THE "DECLARATION OF COVENANTS AND 90. "OAJ" CAP CONDITIONS AND RESTRICTIONS" AS RECORDED IN "WESTWOOD 4th REPLAT". DRAWN: RIK L. SANDERS WITNESS MY HAND AND SEAL THIS BY DAY OF Movem But 2015. (ALL DIMENSIONS ARE FIELD AND RECORD) S00'13'27"W 83.76 WESTWOOD 60' RC S88'36'19"W 417.94' **LEGEND** BLOCK 7A "WOODGREST 5/8"x30" REBAR AND CAP N89'46'30"W 309.05' VILLAS A SET THIS SURVEY OF BREEDEN INVESTMENT GROUP, INC. REPLAT OF O FOUND 5/8" REBAR BLOCK 7A" BEFORE ME, THE UNDERSIGNED NOTARY PUBLIC, IN AND FOR THE COUNTY AND STATE, PERSONALLY APPEARED MARK A. PRAIT, PRESIDENT OF BREEDEN INVESTMENT GROUP, INC. AND EACH SEPARATELY AND SEVERALLY ACKNOWLEDGED THE EXECUTION OF THE FOREGOING INSTRUMENT AS THEIR VOLUNTARY ACT AND DEED FOR THE ☐ FOUND INDOT CONC. ROW MARKER P8 "K", P103 DETENTION EASEMENT "OAJ" ORIGINAL PLAT MONUMENT WITNESS MY HAND AND NOTARIAL SEAL THIS S89'46'30"E 256.94' PROPERTY DESCRIPTION BLOCK A1 AS SHOWN ON THE PLAT OF "WESTWOOD 4th REPLAT" RECORDED IN PLAT BOOK "R", PAGE NOTARY PUBLIC 483.33° OFFICE OF THE RECORDER OF BARTHOLOMEW COUNTY, INDIANA. x The Conseed BLOCK A-B CURRENT OWNER OF RECORD: COUNTY OF RESIDENCE WDIANA MINIMUM 4.73 ACRES BREEDEN INVESTMENT GROUP, INC. AUD. PARCEL ID: NO2'06'5 #03-95-28-110-000,500-005 MY COMMISSION EXPIRES #03-95-28-120-000.106-005 CHORD LENGTH CHORD BEARING DELTA ANGLE TANGENT CURVE RADIUS ARC LENGTH 30.00 87*28'17" 28.70 C1 45.80' 41.48 S43°57'49"W SITE LOCATION MAP STATE ROAD #46 SCALE: 1"=500" LOT 6 BLOCK A-B PINE RIDGE DRIVE N89'00'59"E 437.60 YARD LINE SURVEYOR'S STATEMENT

I, ENOCH R. GRAY, III HEREBY CERTIFY THAT I AM A PROFESSIONAL LAND SURVEYOR, LICENSED IN COMPLIANCE WITH THE LAWS OF THE STATE OF INDIANA; THAT, TO THE BEST OF MY KNOWLEDGE, THIS PLAT CORRECTLY REPRESENTS A SURVEY COMPLETED BY ME IN NOVEMBER OF 2015; THAT ANY CHANGES FROM THE DESCRIPTION APPEARING ON THE LAST RECORD TRANSFER OF THE LAND CONTAINED IN THE FINAL PLAT ARE SO INDICATED; THAT ALL MONUMENTS SHOWN THEREON ACTUALLY EXIST OR WILL BE INSTALLED. I MAKE NO REPRESENTATION, EXPRESSED OR IMPLIED, AS TO THE SUITABILITY OF THIS REAL ESTATE FOR THE CONSTRUCTION OF BASEMENTS.

STATE OF SIAIL SIAIL WOLANA WORKS PRO-FORMA

I AFFIRM, UNDER THE PENALTIES FOR PERJURY, THAT I HAVE TAKEN REASONABLE CARE TO REDACT EACH SOCIAL SECURITY NUMBER IN THIS

DOCUMENT, UNLESS REQUIRED BY LAW. ENOCH R. GRAY, III

NOVEMBER 24, 2015

DATE

LS-0516

ENOCH R. GRAY, III

PRIMARY APPROVAL

UNDER AUTHORITY PROVIDED BY IC 36-7-4-700, SUBDIVISION CONTROL, AND ANY AMENDMENTS THERETO. THIS PLAT WAS GIVEN PRIMARY APPROVAL BY THE CITY OF COLUMBUS, INDIANA AS FOLLOWS: APPROVED BY THE CITY PLAN COMMISSION AT A MEETING HELD DECEMBER 09, 2015

CHAIRMAN -					
SECRETARY -					
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AUDITOR'S CERTIFICATE

THE REAL PROPERTY	has been duly	ENTERED FOR TAXATION AND) Transferred
ON THE RECORDS OF	THE AUDITOR OF	BARTHOLOMEW COUNTY,	

THIS	DAY OF		2015.
-		•	

BARBARA J. HACKMAN, BARTHOLOMEW COUNTY AUDITOR

RECORDING CERTIFICATE

RECORDED IN PLAT BOOK	"R", PAGE		THIS _	 	DAY
OF		2015 AT		O,CTOCK	M.

. FEE PAID .

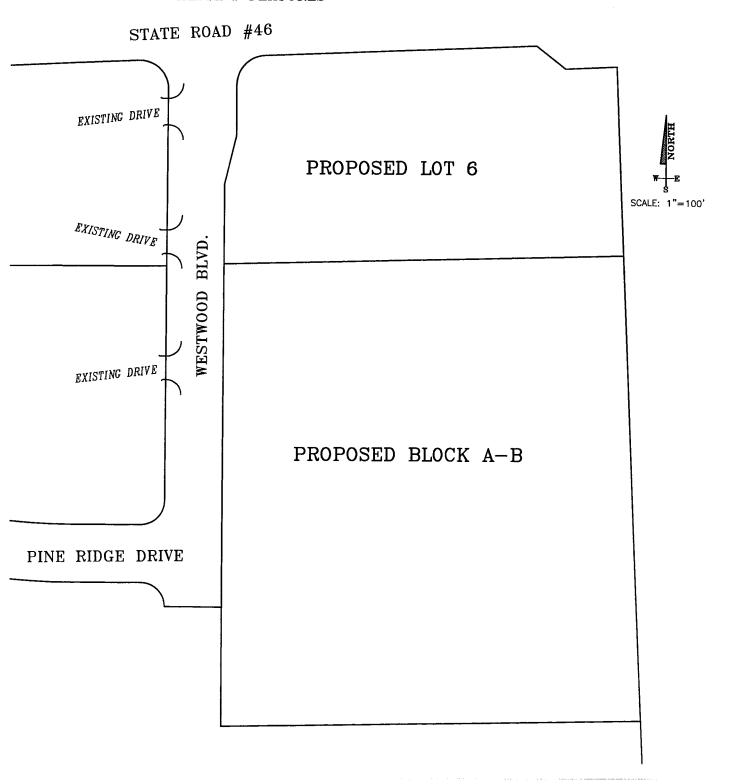
ANITA L. HOLE, BARTHOLOMEW COUNTY RECORDER

A NOTATION HAS BEEN MADE ON THE ORIGINAL PLAT OF "WESTWOOD 3rd REPLAT" RECORDED IN PLAT BOOK "R", PAGE

SURVEYOR'S REPORT RECORDED IN

JOB #15383 E.R. GRAY ASSOCIATES PROFESSIONAL LAND SURVEYING AND CONSULTING E.R. GRAY III, L.S. P.O. BOX 1357 COLUMBUS, INDIANA 47202 BUS. 812-372-7398 FAX 812-372-2175

EXISTING FEATURES





TRAFFIC IMPACT STUDY

For
Proposed Ricker Oil Company Development
Columbus, Indiana

Prepared For:
Ricker Oil Company
And
The City of Columbus, Indiana

Prepared By:
Infrastructure Engineering, Inc.
2601 Fortune Circle East, Suite 202B
Indianapolis, Indiana 46241

February 5, 2016

Traffic Impact Study

For

Proposed Ricker's Development Columbus, Indiana

I certify that this Traffic Impact Study has been prepared by me or under my immediate supervision and that I have experience and training in the field of traffic and transportation engineering.

Dustin T. Qu

in T. Quincy, P.E.

Jane Canada, E.I.



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Executive Summary

Study Purpose

This traffic impact study (TIS) is meant to assess the level of service (LOS) impacts caused by the Ricker's gas station and convenience store proposed for the southeast quadrant of the intersection of SR 46 (Johnathan Moore Pike) and Westwood Boulevard, to the south, and County Road North 325 West, to the north in Columbus, Indiana (Figure 1). This study is also meant to provide recommendations for any necessary improvements required to maintain an acceptable level of service on existing Westwood Boulevard. These recommendations are based on both opening day 2016 traffic volumes and projected traffic volumes in the year 2036. This executive summary is meant to provide an overview of the following detailed report. For traffic collection approach, analysis methodology and additional site related details proceed to the body of the report.

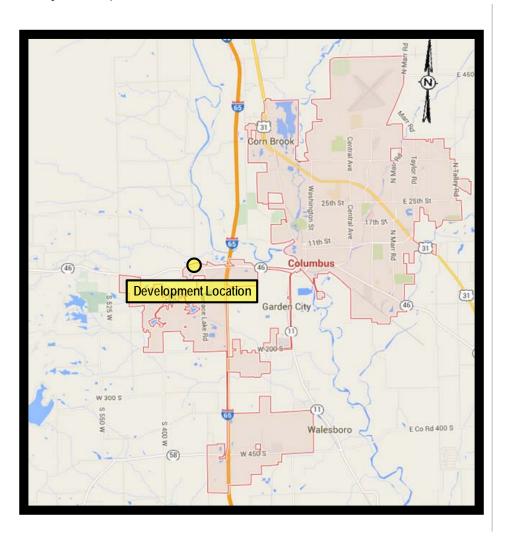


Figure 1 – Study Location

Study Scope Summary

This study focuses on the intersection of SR 46 and Westwood Boulevard to the south and County Road North 325 West to the north. Adjacent intersections along SR 46 are currently functioning at a high levels of service and therefore will not be analyzed as part of this report.

In order to analyze the effects of the Ricker's site on the SR 46 and Westwood Blvd and N 325 W at this intersection, 4 scenarios were examined:

- Scenario 1 Year 2016 existing AM and PM peak hour traffic without the Ricker's development.
- Scenario 2 Year 2016 existing AM and PM peak hour traffic with the proposed Ricker's development.
- Scenario 3 Year 2036 future AM and PM peak hour traffic without the Ricker's development.
- Scenario 4 Year 2036 future AM and PM peak hour traffic with the proposed Ricker's development.

These scenarios show the effects of the proposed Ricker's on both opening day in 2016 and its forecasted effects in year 2036. Recommendations for improvements which will maintain an acceptable level of service for the aforementioned intersection are derived from the analysis of these situations. It must be noted that these are only recommendations and no design plans or specifications regarding the improvements are provided with this report.

Proposed Development Summary

The proposed site is a Ricker's gas station which contains 16 passenger vehicle pumps and a convenience store. The resulting property size will be 2.08 acres when development is complete. The location and overall layout of this proposed Ricker's gas station and convenience store can be seen in Figure 2.

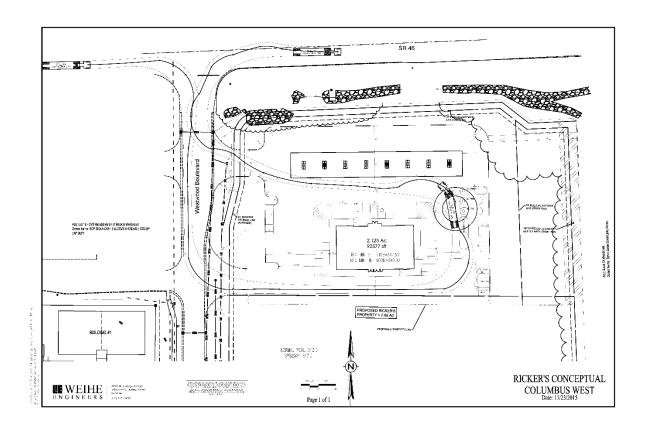


Figure 2 - Proposed Development Layout

Traffic Summary

2014 Average Annual Daily Traffic (AADT) volumes for the SR 46 were obtained through INDOT's Traffic Count Database System (TSDS) and then projected to 2016 using the 2014 annual growth rate. This AADT was used with approval from the City of Columbus Engineering Department. 48 hour counts for Westwood Boulevard and N 325 W and were collected by Infrastructure Engineering, Inc. (IEI) using mechanical counters from the evening of Tuesday, January 26, 2016 to the morning of Friday, January 29, 2016. Turning movements for all approaches were collected by IEI during the AM peak hours of 6am to 8am on January 28, 2016 and for the PM peak hours of 4pm to 6pm on January 26, 2016. Forecasted 2036 volumes were projected using a growth rate of 1% which was provided to IEI by the City of Columbus's engineering department. The trips generated by the proposed Ricker's site were calculated using the appropriate tables, plots and equations found in the Institute of Transportation Engineers' *Trip Generation Manual*, *9th Edition*.

Findings and Recommendations Summary

After completing the analysis of the study intersection it was found that the Ricker's development will have little impact on the level of service for SR 46, Westwood Boulevard and N 325 W. The existing turn lanes on all approaches have adequate storage capacity and the signal timing does not need to be adjusted for this development.

Background Information

Project Scope and Purpose

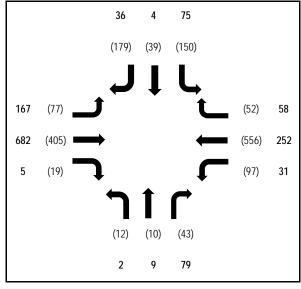
This traffic impact study (TIS) is meant to assess the level of service (LOS) impacts caused by the Ricker's gas station and convenience store proposed for the southeast quadrant of the intersection of SR 46 (Johnathan Moore Pike) and Westwood Boulevard, to the south, and County Road North 325 West, to the north in Columbus, Indiana (Figure 1). By analyzing the intersection of Westwood Boulevard/N 325 W and SR 46 using both the existing 2016 traffic volumes and the projected 2036 volumes a reasonable conclusion can be made regarding the effects of this development. This study will also make recommendations as to whether or not geometric or signal improvements need to be made to maintain an acceptable level of service at this intersection.

Existing Roadway Conditions within Study Area

The intersection being studied is that of SR 46 and Westwood Boulevard/County Road North 325 West. SR 46 runs east and west on the northern edge of the proposed site with a speed limit of 45 mph. East of the intersection, SR 46 consists of one westbound thru lane with one dedicated left turn lane and one dedicated right turn lane. The east side of SR 46 also has two eastbound thru lanes and a two foot raised median separating the east and west bound lanes. West of the intersection SR 46 consists of one westbound thru lane and two eastbound thru lanes. The west side of SR 46 also contains one dedicated left turn lane and one dedicated right turn lane. There is no raised median west of the intersection.

Westwood Boulevard runs north and south along the west edge of the proposed site with a speed limit of 30 mph. Westwood Boulevard is a curbed two lane road. The northbound through lane becomes the dedicated left turn lane. The thru and right turn lane is an auxiliary lane with 145 feet of storage with a 50 foot taper. North of the intersection Westwood Boulevard becomes Country Road North 325 West and has a 45 mph speed limit. N 325 W is a two lane uncurbed road with a dedicated right turn lane. This intersection is currently signalized with no dedicated phasing for turning movements. The intersection is currently functioning an overall A Level of Service in the AM peak hour and a B LOS for the PM peak hour. Figure 3 shows the 2016 turning movements without the proposed Ricker's gas station and convenience store.

SR 46 & Westwood Bouldard / N 325 W



AM Rates PM Rates



Figure 3 - 2016 Traffic Turning Movements and Flow Rates without Ricker's Development

Existing Land Use within Study Area

The existing site is a vacant, grassy area. The current property size is 2.125 acres. There is no existing access to the property via state, city or privately maintained roadways.

Directly across from the proposed site is an existing CVS Pharmacy. South of the CVS there is a new commercial development under construction. Less than 1000 feet south of the proposed Ricker's is a large apartment complex called Westwood Pines. Westwood Boulevard dead-ends into the entrance of this apartment complex and tees with Pine Ridge Drive which continues to the west.

In the northeast quadrant of the intersection of SR 46 and Westwood Boulevard/N325W there is a large development that holds a grocery store, fast-food restaurants, retail space and two banks. In the northwest quadrant of the intersection there is a small veterinary clinic and currently abandoned office buildings. Beyond these developments, the area north of the intersection is largely rural which a few scattered residences.

Proposed Development Description

The proposed site is a Ricker's gas station and convenience store which contains 16 passenger vehicle pumps and a convenience store. The resulting property size will be 2.08 acres when development is complete. There are two entrance/egress drives located on the east side of Westwood Boulevard. Both drive access points to the site line up directly with the access drives to the CVS. The first entrance/egress drive is located less than 100 feet south of the existing signalized intersection of Westwood Boulevard and SR 46. The location and overall layout of this proposed Ricker's gas station and convenience store can be seen in Figure 2.

Between the proposed Ricker's development and Westwood Pines there are several acres that could be developed by a third party, as well as the commercial development currently under construction south of CVS. This study will not take into account the future traffic that may be generated by the future development of these parcels.

Traffic Forecasting

Traffic Data and Collection Methods

2014 Average Annual Daily Traffic (AADT) volumes for the SR 46 were obtained through INDOT's Traffic Count Database System (TSDS) and then projected to 2016 using the INDOT forecated 2014 annual growth rates. For the east bound positive lanes the growth factor is 2% and for the west bound lanes the growth factor is -11%. This AADT was used with approval from the City of Columbus Engineering Department. 48 hour counts for Westwood Boulevard and N 325 W and were collected by IEI using mechanical counters from the evening of January 26, 2016 to the morning of January 29, 2016. Turning

movements for all approaches were collected by IEI during the AM peak hours of 6am to 10am on January 28, 2016 and for the PM peak hours of 4pm to 6pm on January 26, 2016. The Jamar Technologies, Inc. Trax Apollyon was used to collect the mechanical counts and the TDC Ultra Traffic Collector was used to get the turning movements. The trips generated by the proposed Ricker's site were calculated using the appropriate tables, plots and equations located in the Institute of Transportation Engineers' *Trip Generation Manual*, 9th Edition. See appendix A, B and C for all collected traffic data.

Trip Generation Methodology

The primary reference document used to forecast the trip generation caused by the Ricker's site was the *Trip Generation Manual:* 9th Edition. This document takes past traffic studies conducted around the United States and Canada, classifies the studies by their specific land use, and then creates statistical tables, equations and plots that allow designers to better predict the trips that a similar site will generate. The statistical data derived from these past traffic studies is utilized by designers through independent variables associated with a particular land use. These independent variables are physically measureable features of a site and can be reasonably correlated to the use of the site. For example, the square footage of a retail clothing store is an independent variable which can be correlated to the amount of customers it attracts and the amount of trips it generates, likewise so are the number or units in an apartment complex. Some land uses may have two or more independent variables that could be reasonably correlated to the number of trips generated. In a case like this the designer must choose the most reasonable independent variable that best forecasts the trips generated by the site.

For this site's particular land use, Gasoline/Service Station with Convenience Market (945), there are two possible independent variables: the convenience store's gross floor area or the number of vehicle fueling positions. In this case, the number of fueling positions was determined to be the more appropriate independent variable because it is the most causal reason for drivers to enter the proposed site. A number of factors contributed to this determination, including:

- *Proximity to the SR 46 and the IR 65 Interstate interchange* The proposed Ricker's station is the only service station located west of the IR 65 to SR 46 interchange. Therefore, it will divert cars from the interstate and SR 46 for its service pumps rather than its convenience store items.
- *Proximity to a CVS Pharmacy location* It is reasonable to assume that most trips diverted to the new Ricker's location will be caused by the service pumps because they will have a choice to purchase similar convenience store items directly across the street at CVS.

Forecasting the number of trips generated by the site simply consists of multiplying the number of service pumps by the average rate listed in the tables found on pages 1995 and 1996 of the *Trip Generation Manual: 9th Edition.* The peak AM hour rate of 10.56 trips per pump was used generating approximately 170 trips into and out of the proposed Ricker's. The PM rate of 13.56 trips per pump was used generating approximately 218 trips into and out of the proposed site. Figure 4 and Figure 5 show the final 2016 opening day generated trips for the site.

Diverted Linked Trip Generation Methodology

Diverted linked trips are trips to the site which do not originate on, or continue along the adjacent roadway. Diverted linked trips for this site are defined as those trips traveling along SR 46 which stop at the site and then continue their primary trip along SR 46 rather than continuing along the site's adjacent roadway, Westwood Blvd/ N 325 W. These trips do add volume to the adjacent roadway because vehicles that would not otherwise be there are utilizing Westwood Boulevard to access the Ricker's site.

The diverted linked trip percentage is derived by utilizing Table 5.29 and 5.30 in the *Trip Generation Manual* which shows diverted link trip survey data on 9 different gas stations classified as #945. By averaging the data in these tables a diverted link trip percentage of 21% is generated for the AM and 31% for the PM hours. Figure 4 and Figure 5 show the 2016 opening day diverted linked trips for this site.

Pass-By Trip Generation Methodology

Pass-by trips are trips to the site which originate from an adjacent roadway and then continue along that same adjacent roadway. These trips are not the driver's final destination but an intermediate stop. In this study, pass-by trips are defined as those trips traveling along Westwood Blvd/ N 325 W which stop at the site and then continue along Westwood Blvd/ N 325 W or SR 46 to their primary destination or vice versa. These trips do not add volume to Westwood Boulevard or SR 46 as they would have already been traveling these roads regardless of the presents of the Ricker's site.

For this site the AM percentage pass-by trips is 62% of the total AM trips generated and the PM percentage used is 56% of the total PM trips generated. These percentages were derived by utilizing Table 5.29 and 5.30 in the *Trip Generation Manual*. Figure 4 and Figure 5 show the 2016 opening day pass-by trips for this site.

Primary Trip Generation Methodology

Primary Trips are trips generated by the site that which originate on the adjacent roadway and have the sole purposed of going to the site and returning to their point of origin. This study defines primary trips as those started and completed on Westwood Boulevard or N 325 W only.

The percentage of primary trips is derived by simply finding the remaining percentage of total trips not classified as diverted link or pass-by. In this case the AM percentage primary trips is 17% and the PM percentage primary trips is 13%. Figure 4 and Figure 5 show the 2016 opening day primary trips for this site.

Trip Distribution Methodology

Trip Distribution describes from which direction each trip to the site enters and exits. Each type of trip has its own trip distribution that best describes how that trip type functions in the adjacent roadway network. In this study it was determined that 90% of the primary trips to the site will enter headed northbound and exit headed southbound at AM and PM peak hours. This percentage trip distribution was chosen to reflect the draw the new Ricker's will have on motorists living in the existing apartment complex and hotel to the south of the site. The apartment complex and hotels is also why it was determined that 90% of pass-by trips will enter and exit heading northbound on Westwood Boulevard in the AM and 90% will enter and exit heading southbound in the PM peak hour. This trip distribution reflects the direction distribution found at the peak hours for trips originating at the apartment complex. Finally, diverted linked trips to the site were determined to have 98% entering traveling southbound on Westwood Boulevard and then exiting northbound at AM and PM peak hours. This percentage is reasonable because SR 46 is the closest and most heavily traveled roadway that can have traffic diverted from it in the context of this study. All final trip distributions for all trip types can be seen in Figures 4 and 5.

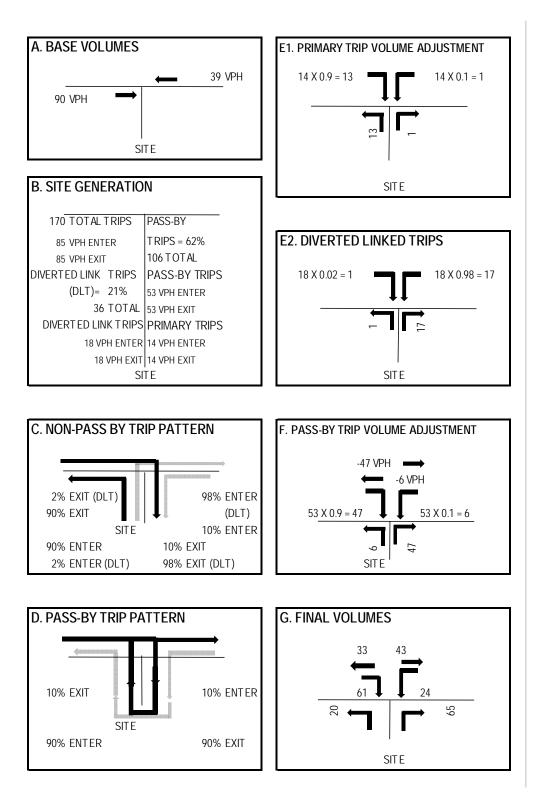


Figure 4 –2016 A.M. Traffic: Diverted Linked Trips, Primary Trips and Pass-By Trips with Ricker's Development

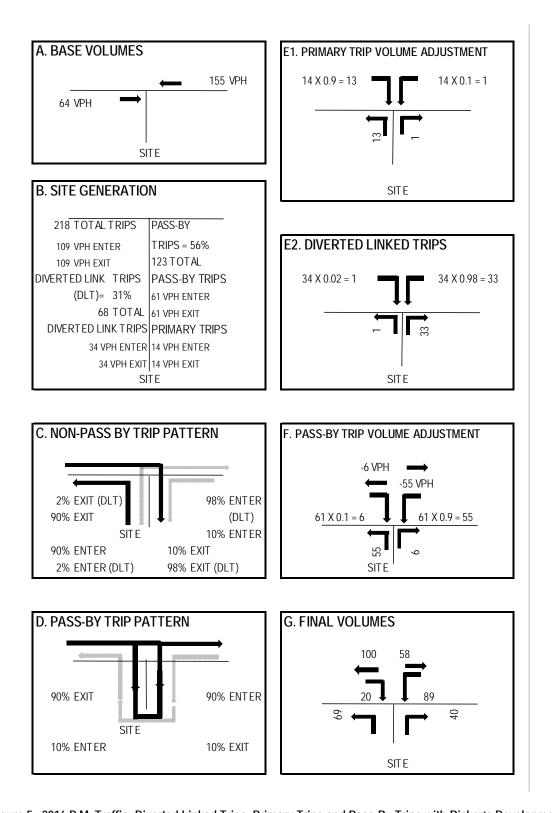


Figure 5 –2016 P.M. Traffic: Diverted Linked Trips, Primary Trips and Pass-By Trips with Ricker's Development

Resulting Forecasted 2036 Traffic Volumes

The opening day traffic counts used for this report were projected to the 2036 traffic volumes by using a growth factor of 1%. This growth factor was recommended by the City of Columbus Engineering Department.

From 2016 to 2036 there will be no increase in the trips generated to the site itself. This is due to the overall methodology used to predict the trips generated by the site. The *Trip Generation Manual's* use of independent variables causes the generated trips to be based only on the site's individual characteristics and not the surrounding traffic volume. Therefore the trips used on opening day 2016 and those in 2036 will be the same because both scenarios assume the Ricker's independent variables are constant and therefore the trips generated as well. The diverted linked trips percentage, pass-by percentage and trip distribution percentages were assumed to remain the same. The future 2036 trip generation information for the site can be seen in Figure 6 and Figure 7.

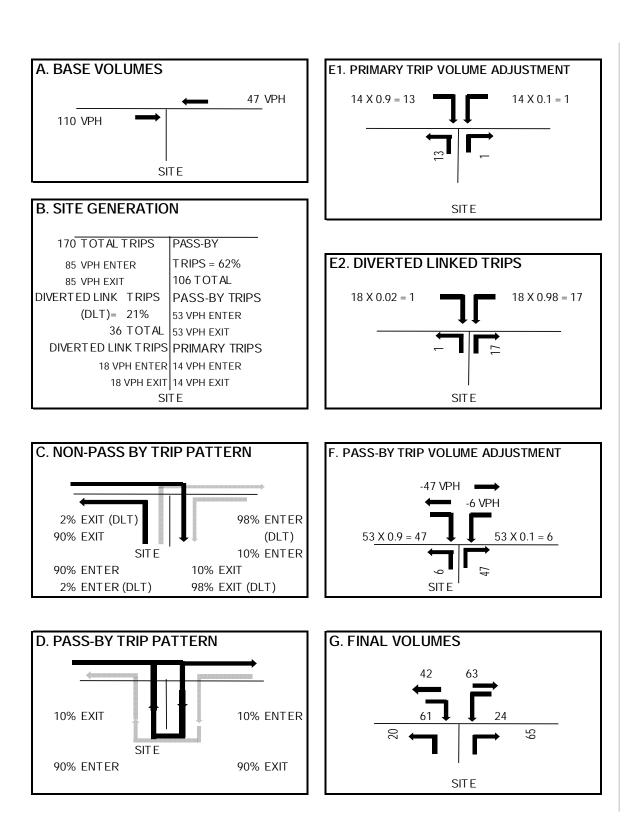


Figure 6 –2036 A.M. Traffic: Diverted Linked Trips, Primary Trips and Pass-By Trips with Ricker's Development

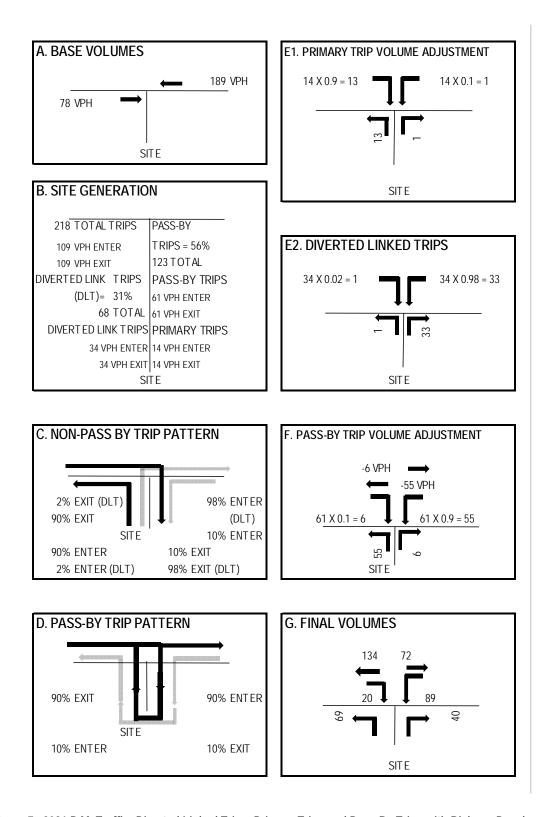


Figure 7 –2036 P.M. Traffic: Diverted Linked Trips, Primary Trips and Pass-By Trips with Ricker's Development

Capacity Analysis

Existing Signal Timing and Turn Lane Configuration Requirements

The intersection of SR 46 and Westwood Boulevard/ N 325 W has an uncoordinated, actuated signal. SR 46 has a maximum green time of 60 seconds and a minimum green time of 10 seconds. Westwood Boulevard and N 325 have a maximum green time of 30 seconds and a minimum green time of 7 seconds. The east-west movements occur simultaneously as do the north-south movements. There are no dedicated turning phases. For a complete summary of the signal timing of this intersection, refer to Appendix F.

The current turn lane configuration was observed to be adequately storing vehicles during both the AM and PM peak hours. The intersection was then run in HCS 2010 to predict whether the turn lanes will continue to experience adequate storage with the addition of the trips generated by the Ricker's development.

Level of Service Requirements

Level of service (LOS) is a means of conveying the operating conditions of an intersection and is directly tied to the amount of delay a driver experiences at an intersection. Delay is used as the primary means of rating an intersection's functionality because it is directly linked to a driver's frustration, fuel consumption, travel delay and overall discomfort. The *Highway Capacity Manual* defines LOS using an A through F scale with A having the least amount of delay and F having the most. The delay related to each letter is shown in Table 1 and is also based on the type of traffic control used at an intersection.

Level of Service	Unsignalized	Signalized
(LOS)	(Seconds of Delay Per Vehicle)	(Seconds of Delay Per Vehicle)
А	0 - 10.0	0 - 10.0
В	10.1 - 15.0	10.1 - 20.0
С	15.1 - 25.0	20.1 - 35.0
D	25.1 - 35.0	35.1 - 55.0
Е	35.1 - 50.0	55.1 - 80.0
F	Over 50	Over 80

Table 1 - Level of Service Parameters

Each direction of travel at an intersection is assessed for its individual level of service. These individual ratings are then averaged in order to achieve an overall LOS for the intersection. For this study the intersection must achieve averaged LOS of a C or better to meet an acceptable level of operation.

Traffic Scenarios

Scenario 1 – Year 2016 existing AM and PM peak hour traffic without the Ricker's site.

This is the baseline scenario analyzed for LOS using the traffic analysis software HCS 2010. This scenario analyzes the existing roadway network in year 2016 assuming that the proposed Ricker's development is not present. Table 2 shows the existing LOS and a view of the existing turning movements can be seen in Figure 3.

	umes I	Pre-De	evelopmer	nt										
Approach		SR 46 - EF	3	SR 46 - WB			Westv	vood Boule	vard - NB	N 325 W - SB				
Approach Movement	Left	Through	Right	Left	Through	Right	Left	Through	Right	Left	Through	Right		
Control Delay, s/veh	5.3	4.1	4.1	4.8	3.8	3.2	32.7	33.1		36.9	32.0			
Level of Service (LOS)	Α	Α	Α	Α	Α	Α	С	С		D	С			
Approach Delay, s/veh		4.3		3.8			33.1			35.2				
Approach Leve of Service		А			Α		С				D			
Intersection Delay, s/veh	8.6							A						
/ Interseciton LOS	ŏ.ō							A						
	20)16 PM - P	eak Ho	ur Tu	rning Vol	umes F	Pre-De	evelopmer	nt					
Approach		SR 46 - E	3	SR 46 - WB			Westv	vood Boule	vard - NB	N 325 W - SB				
Approach Movement	Left	Through	Right	Left	Through	Right	Left	Through	Right	Left	Through	Right		
Control Delay, s/veh	10.7	5.2	5.2	6.6	7.5	4.6	41.0	30.0		35.1	36.6			
Level of Service (LOS)	В	Α	Α	Α	Α	Α	D	С		D	D			
Approach Delay, s/veh		6.0			7.2			32.0			36.0			
Approach Leve of Service		Α			А		С			D				
Intersection Delay, s/veh			1/	1.3					В					
/ Interseciton LOS			14	r.3			В							

Table 2 – Existing Level of Service Year 2016 without Development

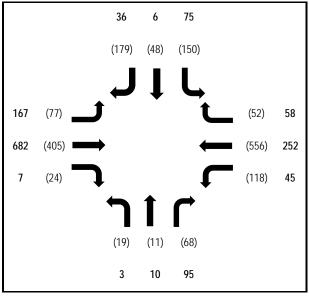
Scenario 2 – Year 2016 existing AM and PM peak hour traffic with proposed Ricker's Development.

This scenario looks at the existing intersection with the proposed Ricker's development using opening day traffic volumes for year 2016. The LOS rating for the intersection is shown in Table 3 and a view of turning movements can be seen in Figure 8.

	umes F	Post-D	evelopme	ent										
Approach		SR 46 - EF	3	SR 46 - WB			West	wood Boul	evard - NB	N 325 W - SB				
Approach Movement	Left	Through	Right	Left	Through	Right	Left	Through	Right	Left	Through	Right		
Control Delay, s/veh	5.7	4.4	4.4	5.4	4.1	3.4	32.4	33.1		37.3	31.6			
Level of Service (LOS)	Α	Α	Α	Α	А	Α	С	С		D	С			
Approach Delay, s/veh		4.6		4.1				33.1		35.3				
Approach Leve of Service		Α			Α		С			D				
Intersection Delay, s/veh	9.2							A						
/ Interseciton LOS								A						
	20	16 PM - P	eak Ho	ur Tu	rning Volu	umes F	ost-D	evelopme	nt					
Approach		SR 46 - EF	3	SR 46 - WB			West	wood Boul	evard - NB	N 325 W - SB				
Approach Movement	Left	Through	Right	Left	Through	Right	Left	Through	Right	Left	Through	Right		
Control Delay, s/veh	11.4	5.6	5.6	6.9	7.1	4.9	41.3	30.1		36.0	36.0			
Level of Service (LOS)	В	Α	Α	Α	Α	Α	D	С		D	D			
Approach Delay, s/veh		6.5			6.9		32.3			36.0				
Approach Leve of Service		Α			Α			С			D			
Intersection Delay, s/veh			1,	1 4			, , , , , , , , , , , , , , , , , , ,							
/ Interseciton LOS			14	1.6					В					

Table 3 – Opening Day Level of Service Year 2016 with Development

SR 46 & Westwood Bouldard / N 325 W



AM Rates (PM Rates)



Figure 8 – 2016 Traffic Turning Movements and Flow Rates with Development

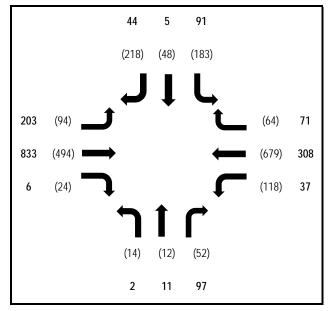
Scenario 3 – Year 2036 existing AM and PM peak hour traffic without the Ricker's site.

This is the baseline scenario analyzed for LOS using the traffic analysis software HCS 2010. This scenario analyzes the existing roadway network in year 2036 assuming that the proposed Ricker's development is not present. Table 4 shows the existing level of service and a view of the existing turning movements can be seen in Figure 9.

	nes Wi	thout	Developn	nent									
Approach		SR 46 - EF	3	SR 46 - WB			West	wood Boul	evard - NB	N 325 W - SB			
Approach Movement	Left	Through	Right	Left	Through	Right	Left	Through	Right	Left	Through	Right	
Control Delay, s/veh	7.3	5.1	5.1	6.5	4.7	3.8	32.3	32.6		37.5	31.3		
Level of Service (LOS)	Α	А	Α	Α	Α	Α	С	С		D	С		
Approach Delay, s/veh		5.6		4.7				32.6		35.3			
Approach Leve of Service		А		В			С		D				
Intersection Delay, s/veh / Interseciton LOS	9.5							A					
	203	6 PM - Pea	ak Hou	r Turr	ning Volur	nes Wi	thout	Developn	nent				
Approach		SR 46 - EF	3	SR 46 - WB			West	wood Boul	evard - NB	N 325 W - SB			
Approach Movement	Left	Through	Right	Left	Through	Right	Left	Through	Right	Left	Through	Right	
Control Delay, s/veh	17.4	6.7	6.7	9.1	10.8	5.7	42.7	28.9		35.2	40.4		
Level of Service (LOS)	В	А	Α	Α	В	Α	D	С		D	D		
Approach Delay, s/veh		8.3			10.2			31.4		38.3			
Approach Leve of Service		А			В		С				D		
Intersection Delay, s/veh / Interseciton LOS	16.8						В						

Table 4 -Level of Service Year 2036 without Development

SR 46 & Westwood Bouldard / N 325 W



AM Rates PM Rates



Figure 9 – 2036 Traffic Turning Movements and Flow Rates without Development

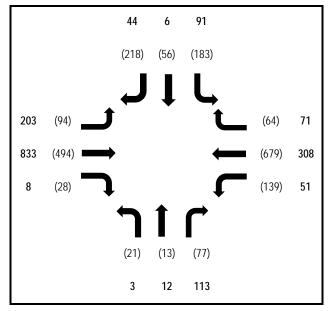
Scenario 4 – Year 2036 existing AM and PM peak hour traffic with proposed Ricker's Development.

This scenario looks at the existing intersection with the proposed Ricker's development using opening day traffic volumes for year 2036. The LOS rating for the intersection is shown in Table 5 and a view of turning movements can be seen in Figure 10.

	2	2036 AM -	Peak H	our T	urning Vo	lumes	With	Developm	ent				
Approach		SR 46 - EF	3	SR 46 - WB			West	wood Boul	evard - NB	N 325 W - SB			
Approach Movement	Left	Through	Right	Left	Through	Right	Left	Through	Right	Left	Through	Right	
Control Delay, s/veh	7.8	5.5	5.5	7.3	5.0	4.1	31.9	32.6		37.9	30.9		
Level of Service (LOS)	Α	Α	Α	Α	А	Α	С	С		D	D		
Approach Delay, s/veh		6.0		5.1				32.6		35.4			
Approach Leve of Service		Α			А			С		D			
Intersection Delay, s/veh / Interseciton LOS			10).1			В						
	2	2036 PM - I	Peak H	our T	urning Vo	lumes	With	Developm	ent				
Approach		SR 46 - EF	3		SR 46 - WI	В	West	wood Boul	evard - NB	N 325 W - SB			
Approach Movement	Left	Through	Right	Left	Through	Right	Left	Through	Right	Left	Through	Right	
Control Delay, s/veh	15.3	6.3	6.3	8.9	9.5	5.4	41.2	29.2		37.2	35.1		
Level of Service (LOS)	В	Α	Α	Α	А	Α	D	С		D	D		
A 151 / 1	7.6 9.1						32.0			35.9			
Approach Delay, s/veh		7.0				А				D			
Approach Leve of Service		A			Α			С			D		

Table 5 – Forecasting Level of Service Year 2036 with Development

SR 46 & Westwood Bouldard / N 325 W



AM Rates PM Rates



Figure 10 – 2036 Traffic Turning Movements and Flow Rates with Development

Summary of Findings and Recommendations

After completing an analysis of the intersection for all of the aforementioned scenarios IEI concludes that the proposed Ricker's site will not aversely effect the intersection enough to warrant any updates to the existing geometrics of the intersection. The signal timing also does not need to be modified to for the opening day or 2036 volumes expected for the Ricker's development.

2016 AM - Peak Hour Turning Volumes Pre-Development												
Intersection	SR 46 - EB	Westwood Boulevard - NB N 325 W - SB										
Intersection Delay, s/veh	8.	4	A									
/ Interseciton LOS	0.	0	A									
	2016 PM - Peak Hour Turning Volumes Pre-Development											
Intersection	SR 46 - EB	SR 46 - WB	Westwood Boulevard - NB	N 325 W - SB								
Intersection Delay, s/veh		2	В									
/ Interseciton LOS	14	٠.٥	D									
2016 AM - Peak Hour Turning Volumes Post-Development												
Intersection	SR 46 - EB	SR 46 - WB	Westwood Boulevard - NB	N 325 W - SB								
Intersection Delay, s/veh	9.	າ	Λ									
/ Interseciton LOS	7.	2	A									
2016 PM - Peak Hour Turning Volumes Post-Development												
Intersection	SR 46 - EB	SR 46 - WB	Westwood Boulevard - NB	N 325 W - SB								
Intersection Delay, s/veh		6	В									
/ Interseciton LOS	14	5.0										
2036 AM - Peak Hour Turning Volumes Without Development												
Intersection	SR 46 - EB	SR 46 - WB	Westwood Boulevard - NB	N 325 W - SB								
Intersection Delay, s/veh	9.	5	A									
/ Interseciton LOS	7.	.5										
	2036 PM - Peak Hou		Vithout Development									
Intersection	SR 46 - EB	SR 46 - WB	Westwood Boulevard - NB	N 325 W - SB								
Intersection Delay, s/veh	16	.8	В									
/ Interseciton LOS												
2036 AM - Peak Hour Turning Volumes With Development												
Intersection	SR 46 - EB	SR 46 - WB	Westwood Boulevard - NB	N 325 W - SB								
Intersection Delay, s/veh	10	11	В									
/ Interseciton LOS	-											
		ur Turning Volumes	•									
Intersection	SR 46 - EB	SR 46 - WB	Westwood Boulevard - NB	N 325 W - SB								
Intersection Delay, s/veh	15	В										
/ Interseciton LOS	10											

Table 6 - Summary of Findings

Appendices

Appendix A – INDOT Traffic Data – SR 46

Appendix B - Collected Traffic Data - Westwood Boulevard & N 325 W

Appendix C – Turning Movements

Appendix D – HCS 2010 Capacity Analysis for Year 2016

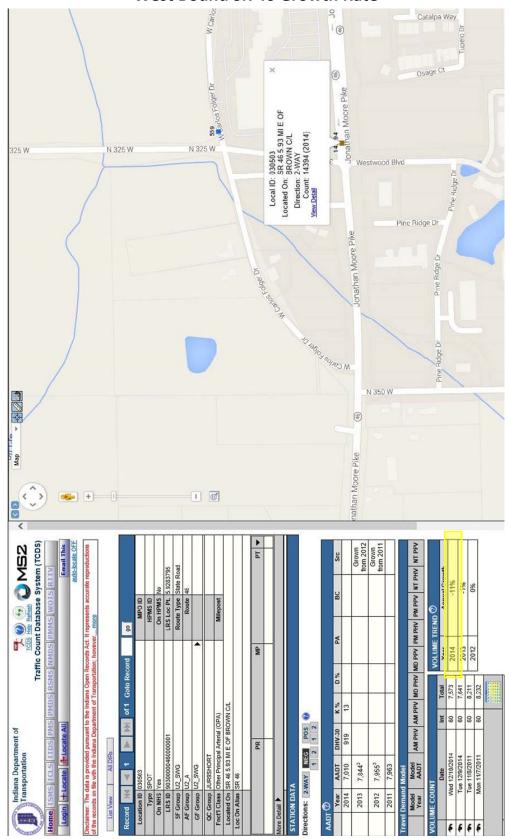
Appendix E - HCS 2010 Capacity Analysis for Year 2036

Appendix F – INDOT Signal Timing

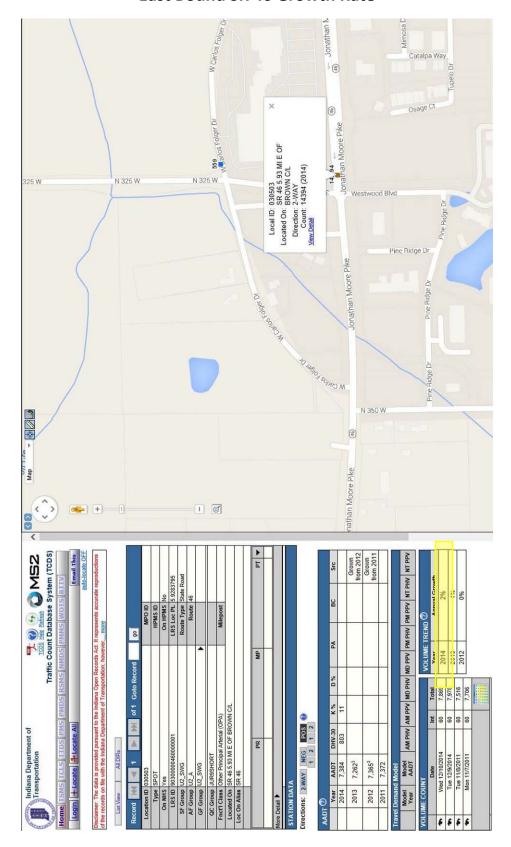


Appendix A - INDOT Traffic Data - SR 46

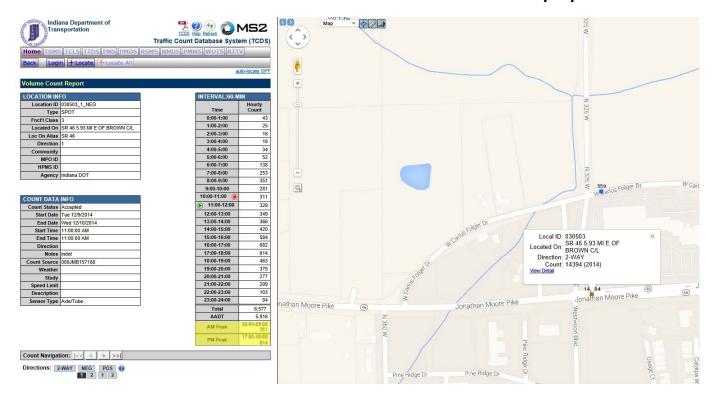
West Bound SR 46 Growth Rate



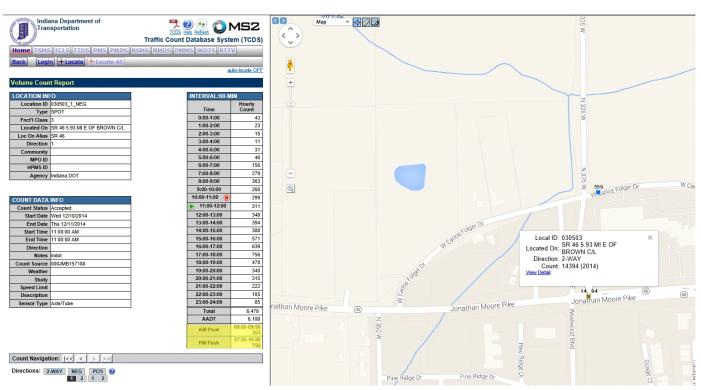
East Bound SR 46 Growth Rate



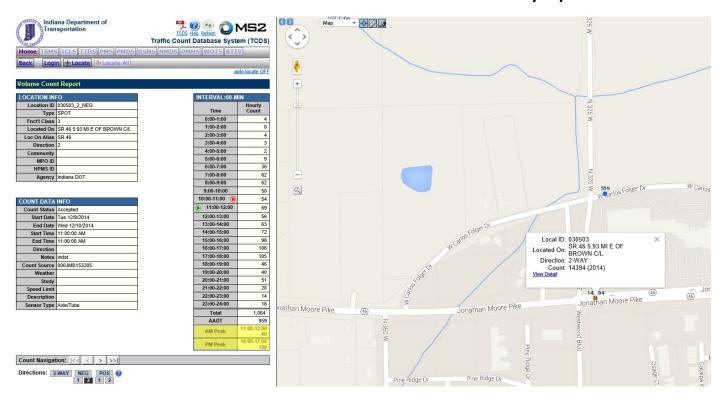
SR 46 West Bound Outside Lane AM and PM Peak Hour Counts 12/09/2014



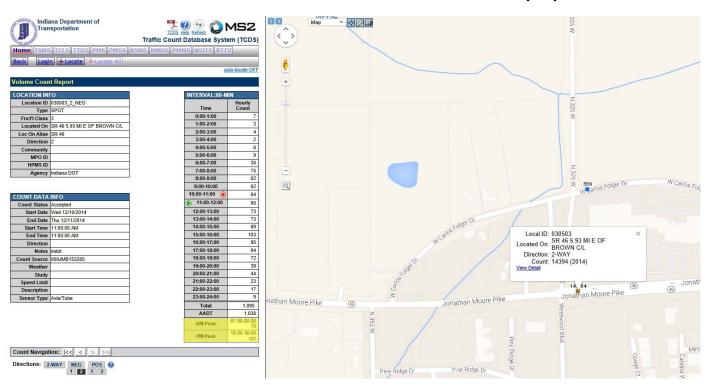
SR 46 West Bound Outside Lane AM and PM Peak Hour Counts 12/10/2014



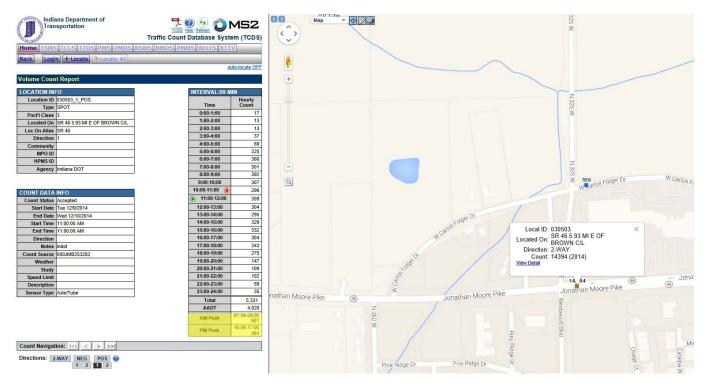
SR 46 West Bound Inside Lane AM and PM Peak Hour Counts 12/09/2014



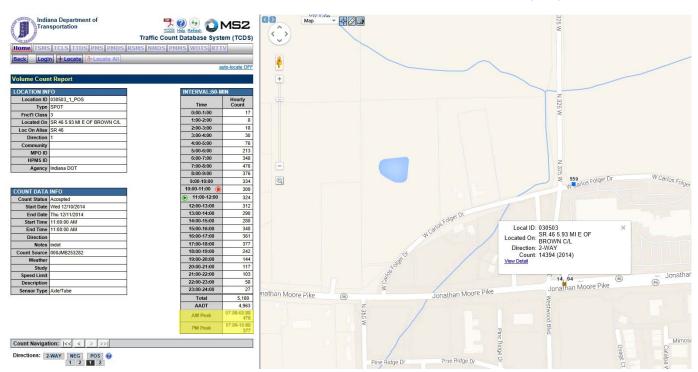
SR 46 West Bound Inside Lane AM and PM Peak Hour Counts 12/10/2014



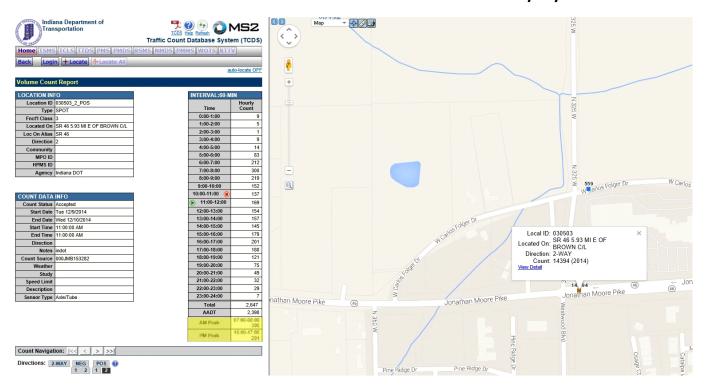
SR 46 East Bound Outside Lane AM and PM Peak Hour Counts 12/09/2014



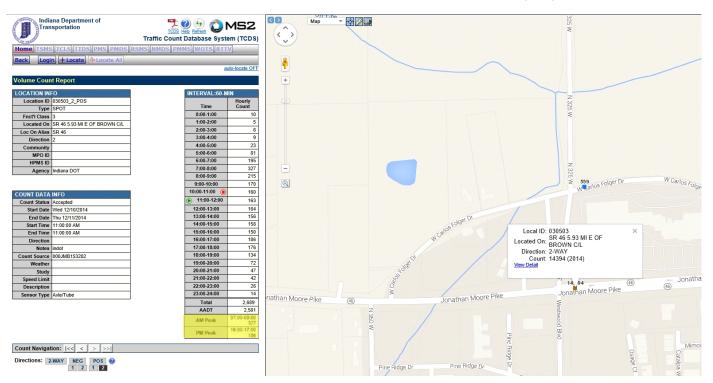
SR 46 East Bound Outside Lane AM and PM Peak Hour Counts 12/10/2014



SR 46 East Bound Inside Lane AM and PM Peak Hour Counts 12/09/2014



SR 46 East Bound Inside Lane AM and PM Peak Hour Counts 12/10/2014





Appendix B - Collected Traffic Data - Westwood Boulevard & N 325 W

Infrastructure Engineering, Inc. 2601 Fortune Circle East, Suite 202B

Indianapolis, IN 46241

Latitude: 39' 12.0320 North Longitude: 85' 58.3950 West

Site Code: SB N 325 W

Start	Mon	Tue	Wed	Thu	Fri	Average	Sat	Sun	Week	
Time	25-Jan-16	26-Jan-16	27-Jan-16	28-Jan-16	29-Jan-16	Day	30-Jan-16	31-Jan-16	Average	
12:00 AM	*	*	10	8	10	9	*	*	9 [
01:00	*	*	4	3	9	5	*		5]	
02:00	*	*	2	8	7	6	*	*	6 🛚	The actual peak
03:00	*	*	14	7	9	10	*	*	10 📗	
04:00	*	*	30	30	28	29	*	*	29 🔲	hour for the enti
05:00	*	*	56	63	69	63	*	*	63	intersection.
06:00	*	*	128	116	135	126	*	*	126	
07:00	*	*	110	115	118	114	*	*	114 🔙	
08:00	*	*	97	90	52	80	*	*	80	
09:00	*	*	80	106	1	62	*	*	62	
10:00	*	*	117	140	*	128	*	*	128	
11:00	*	*	140	147	*	144	*	*	144	
12:00 PM	*	*	140	150	*	145	*	*	145	
01:00	*	*	139	130	*	134	*	*	134	
02:00	*	*	224	191	*	208	*	*	208	
03:00	*	*	267	277	*	272	*	*	272	
04:00	*	*	372	364	*	368	*	*	368	
05:00	*	*	194	194	*	194	*	*	194	
06:00	*	*	130	117	*	124	*	*	124	
07:00	*	71	89	91	*	84	*	*	84	
	*				*		*	*		
08:00	*	52	77	62	*	64	*	*	64	
09:00	*	27	29	40	*	32	*	*	32	
10:00	*	17	19	23	*	20	*	*	20	
11:00		6	4	14		8			8]	
Day Total	0	173	2472	2486	438	2429	0	0	2429	
% Avg.	0.0%	7.1%	101.8%	102.3%	18.0%					
WkDay										
% Avg.	0.0%	7.1%	101.8%	102.3%	18.0%	100.0%	0.0%	0.0%		
Week									11.00	
AM Peak	-	-	11:00	11:00	06:00	- 11:00		-	- 11:00	-
Vol.	-	- 40.00	140	147	135	- 144		-	- 144	
PM Peak	-	19:00	16:00	16:00	-	- 16:00		-	- 16:00	
Vol.	-	71	372	364	-	- 368		-	- 368	
Grand	0	173	2472	2486	438	2429	0	0	2429	
Total	O	1,0	2.,,2	2.00	100	2127	O	J	2,2,	

Infrastructure Engineering, Inc. 2601 Fortune Circle East, Suite 202B

Indianapolis, IN 46241

Latitude: 39' 11.9300 North Longitude: 85' 58.4010 West

Site Code: NB Westwood Blvd

Start	Mon	Tue	Wed	Thu	Fri	Average	Sat	Sun	Week
<u>Time</u> 12:00 AM	25-Jan-16 *	26-Jan-16 *	27-Jan-16 2	28-Jan-16	29-Jan-16 3	Day 2	30-Jan-16 *	31-Jan-16 *	Average 2
01:00	*	*	1	2	4	2	*	*	2
01:00	*	*	2	0	0	1	*	*	1
02:00	*	*	0	1	2	1	*	*	1]
03.00	*	*	6	2	5	4	*	*	4 🗓
05:00	*	*	16	15	16	16	*	*	16
06:00	*	*	49	49	39	46	*	*	46
07:00	*	*		82	92	90	*	*	90
08:00	*	*	68	79	60	69	*	*	69
09:00	*	*	40	46	25	37	*	*	37
10:00	*	*	27	55	*	41	*	*	41
11:00	*	*	46	55	*	50	*	*	50
12:00 PM	*	*	54	46	*	50	*	*	50
01:00	*	*	59	47	*	53	*	*	53
02:00	*	*	44	36	*	40	*	*	40
03:00	*	*	54	46	*	50	*	*	50
04:00	*	*	66	62	*	64	*	*	64
05:00	*	*	55	54	*	54	*	*	54
06:00	*	*	50	60	*	55	*	*	55
07:00	*	33	34	40	*	36	*	*	36
08:00	*	17	18	21	*	19	*	*	19
09:00	*	17	13	14	*	15	*	*	15
10:00	*	7	8	12	*	9	*	*	9 🔲
11:00	*	6	7	4	*	6	*	*	6
Day Total	0	80	814	829	246	810	0	0	810
% Avg. WkDay	0.0%	9.9%	100.5%	102.3%	30.4%				
% Avg. Week	0.0%	9.9%	100.5%	102.3%	30.4%	100.0%	0.0%	0.0%	
AM Peak	-	-	07:00	07:00	07:00	- 07:00		-	- 07:00 -
Vol.	-	-	95	82	92	- 90		_	- 90 -
PM Peak	-	19:00	16:00	16:00	-	- 16:00		-	- 16:00 -
Vol.	-	33	66	62	-	- 64		-	- 64 -
Grand Total	0	80	814	829	246	810	0	0	810

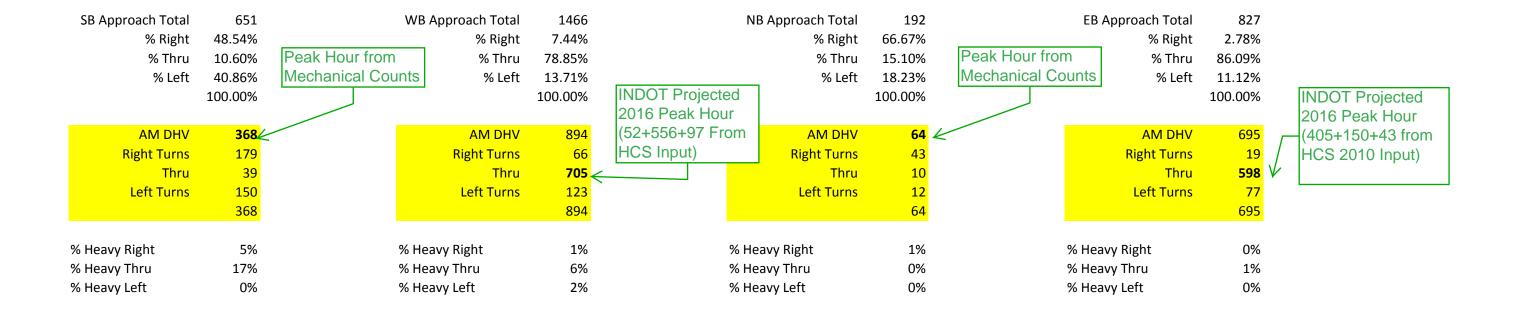
AADT 809



Appendix C – Turning Movements

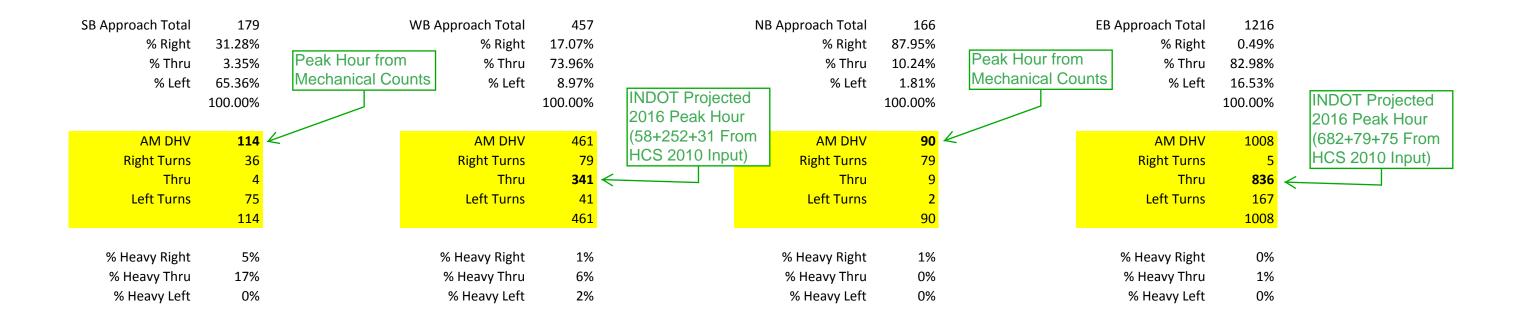
2601 Fortune Circle East, Suite 202B Indianapolis, IN 46241 PM Counts Time: 4:00 PM to 6 PM Date: 1/26/2016

N 35 W (SB Approach)							SR 46 (WB	Approach)		Westwood Boulevard (NB Approach) From South					SR 46 (EB Approach) From West					
From North						From	East													
Start Time	Righ	t Th	ru	Left	Peds	Right	Thru	Left	Peds	Right	Thru	Left	Peds	Right	Thru	Left	Peds			
	4:00 PM	32	6		31	0	16	133	21	0	21	2	4	0	5	56	6	0		
	4:15 PM	39	6		33	0	17	128	21	1	22	5	5	0	2	92	11	0		
	4:30 PM	40	8		25	0	16	139	27	0	7	1	5	0	2	104	11	0		
	4:45 PM	32	7		37	0	17	163	20	0	13	4	5	0	1	97	20	0		
	5:00 PM	51	15		42	0	15	159	33	0	15	4	4	0	2	108	17	0		
	5:15 PM	52	8		49	0	7	156	27	0	18	7	6	0	4	86	7	0		
	5:30 PM	37	12		28	0	12	161	35	0	17	3	2	0	4	87	10	0		
	5:45 PM	33	7		21	0	9	117	17	0	15	3	4	0	3	82	10	0		



2601 Fortune Circle East, Suite 202B Indianapolis, IN 46241 AM Counts Time: 6:00 AM to 8 AM Date: 1/28/2016

N 35 W (SB Approach)								SR 46 (WB A	pproach)		Westwood Boulevard (NB Approach)					SR 46 (EB Approach)					
		From North						From East			From South				From West						
Start Tir	ne	Right	Thru	Left	Peds	Right	Thru	Left	Peds	Right	Thru	Left	Peds	Right	Thru	Left	Peds				
	6:00 AM		1	0	12	0	2	21	1	0	4	0	0	0	0	87	9	0			
	6:15 AM		1	0	7	0	3	21	0	0	16	0	0	0	0	77	8	0			
	6:30 AM		5	1	16	0	6	24	2	0	18	4	0	0	2	110	14	0			
	6:45 AM		5	1	10	0	6	47	3	0	14	2	0	0	0	129	20	0			
	7:00 AM		6	1	15	0	12	48	3	0	10	1	1	0	2	130	38	0			
	7:15 AM		5	1	18	0	14	46	6	0	26	5	0	0	1	160	43	0			
	7:30 AM		15	1	27	0	16	56	7	0	34	2	2	0	1	174	36	0			
	7:45 AM		18	1	12	0	19	75	19	0	24	3	0	0	0	142	33	0			





Appendix D – HCS 2010 Capacity Analysis for Year 2016

HCS 2010 Signalized Intersection Input Data 14. **General Information Intersection Information** Agency Infrastructure Engineering, Inc. Duration, h 0.25 Analyst Jane Canada Analysis Date 1/11/2016 Area Type Other PHF 1.00 Jurisdiction Time Period Intersection SR 46 & Westwood Boulev Analysis Year 2016 **Analysis Period** 1> 7:00 AM Existing 2016 - Ricker's @ SR 46 & Westwood Boulevard - Columbus, IN.xus File Name **Project Description** EΒ WB **Demand Information** NB SB Approach Movement L R L R L R R 167 682 58 Demand (v), veh/h 5 31 252 2 9 79 75 4 36 **Signal Information** ٨, Cycle, s 82.0 Reference Phase 2 Offset, s 0 Reference Point Begin Green 10.8 60.0 0.0 0.0 0.0 0.0 Uncoordinated Yes Simult. Gap E/W On Yellow 3.9 4.3 0.0 0.0 0.0 0.0 Force Mode Fixed Simult. Gap N/S On Red 1.5 1.5 0.0 0.0 0.0 0.0 **Traffic Information** EΒ WB NB SB Approach Movement R Т R R L Τ L L Τ L R Demand (v), veh/h 167 682 5 252 58 9 79 75 36 Initial Queue (Qb), veh/h 0 0 0 0 0 0 0 0 0 0 0 Base Saturation Flow Rate (so), veh/h 1900 1900 1900 1900 1900 1900 1900 1900 1900 1900 1900 1900 Parking (Nm), man/h None None None None Heavy Vehicles (PHV), % 0 1 2 6 1 0 0 0 17 Ped / Bike / RTOR, /h 0 0 0 0 0 0 0 0 0 0 0 0 Buses (Nb), buses/h 0 0 0 0 0 0 0 0 0 0 0 0 3 3 3 3 3 3 3 3 3 3 3 3 Arrival Type (AT) 1.00 1.00 1.00 Upstream Filtering (I) 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 Lane Width (W), ft 12.0 12.0 12.0 12.0 12.0 12.0 12.0 12.0 12.0 100 310 200 1150 750 160 195 290 Turn Bay Length, ft 313 Grade (P_g) , % 0 0 0 0 0 0 0 0 0 0 0 0 Speed Limit, mi/h 45 45 45 45 45 45 35 35 35 40 40 40 **Phase Information** WBL WBT NBT SBL SBT **EBL EBT** NBL Maximum Green (G_{max}) or Phase Split, s 60.0 60.0 30.0 30.0 4.3 Yellow Change Interval (Y), s 4.0 4.0 4.3 4.0 3.9 4.0 3.9 Red Clearance Interval (Rc), s 1.0 1.5 1.0 1.5 1.0 1.5 1.0 1.5 Minimum Green (Gmin), s 5 10 5 10 5 7 5 7 Start-Up Lost Time (It), s 2.0 2.0 2.0 2.0 2.0 2.0 2.0 2.0 Extension of Effective Green (e), s 2.0 2.0 2.0 2.0 2.0 2.0 2.0 2.0 Passage (PT), s 2.0 3.0 2.0 3.0 2.0 1.5 2.0 1.5 Recall Mode Min Max Min Max Min Min Min Min **Dual Entry** No Yes No Yes No Yes Nο Yes 0.0 Walk (Walk), s 0.0 0.0 0.0 0.0 0.0 0.0 0.0 Pedestrian Clearance Time (PC), s 0.0 0.0 0.0 0.0 0.0 0.0 0.0 0.0 **Multimodal Information** EΒ WB NB SB 85th % Speed / Rest in Walk / Corner Radius 25 0 25 0 25 0 25 0 No No No Nο 9.0 0 9.0 0 9.0 12 0 9.0 12 0 Walkway / Crosswalk Width / Length, ft 12 12 Street Width / Island / Curb 0 0 0 0 0 0 0 0 No No No No Width Outside / Bike Lane / Shoulder, ft 12 5.0 2.0 12 5.0 2.0 12 5.0 2.0 12 5.0 2.0

Pedestrian Signal / Occupied Parking

No

0.50

No

No

0.50

0.50

0.50

HCS 2010 Signalized Intersection Results Summary Intersection Information 14. 4177 **General Information** Agency Infrastructure Engineering, Inc. Duration, h 0.25 Analyst Jane Canada Analysis Date 1/11/2016 Area Type Other PHF 1.00 Jurisdiction Time Period Intersection SR 46 & Westwood Boulev Analysis Year 2016 **Analysis Period** 1> 7:00 AM Existing 2016 - Ricker's @ SR 46 & Westwood Boulevard - Columbus, IN.xus File Name **Project Description** WB **Demand Information** EB NB SB Approach Movement R L R R R 682 58 Demand (v), veh/h 167 5 31 252 2 9 79 75 4 36 **Signal Information** ؞؞ڶڶ؞ Cycle, s 82.0 Reference Phase 2 Offset, s 0 Reference Point Begin 60.0 0.0 0.0 0.0 Green 10.8 0.0 Uncoordinated Yes Simult. Gap E/W On Yellow 3.9 4.3 0.0 0.0 0.0 0.0 Force Mode Fixed Simult. Gap N/S 0.0 On Red 1.5 1.5 0.0 0.0 0.0 **Timer Results EBL EBT WBL** WBT NBL **NBT** SBL SBT **Assigned Phase** 4 8 2 6 Case Number 6.0 5.0 6.0 6.0 Phase Duration, s 65.8 65.8 16.2 16.2 Change Period, (Y+Rc), s 5.8 5.8 5.4 5.4 Max Allow Headway (MAH), s 4.1 4.1 2.8 2.8 Queue Clearance Time (gs), s 10.3 8.1 6.0 10.5 Green Extension Time (ge), s 5.0 5.0 0.3 0.2 Phase Call Probability 1.00 1.00 1.00 1.00 0.00 0.00 0.00 0.00 Max Out Probability WB NB SB **Movement Group Results** EΒ Approach Movement L Т R L Т R L Т R L Т R **Assigned Movement** 7 4 14 3 8 18 5 2 12 1 6 16 Adjusted Flow Rate (v), veh/h 167 344 343 31 252 58 2 88 75 40 1086 1881 1876 752 1792 1594 1389 1636 1330 1398 Adjusted Saturation Flow Rate (s), veh/h/ln 4.7 4.9 1.2 3.6 0.1 4.0 4.5 2.1 Queue Service Time (gs), s 4.9 8.0 Cycle Queue Clearance Time (gc), s 8.3 4.9 4.9 6.1 3.6 8.0 2.2 4.0 8.5 2.1 1374 1371 592 237 218 833 1310 1165 199 186 Capacity (c), veh/h Volume-to-Capacity Ratio (X) 0.200 0.250 0.250 0.052 0.192 0.050 0.008 0.405 0.376 0.215 Available Capacity (ca), veh/h 833 1374 1371 592 1310 1165 560 598 508 511 Back of Queue (Q), veh/ln (50th percentile) 8.0 1.3 1.3 0.2 0.9 0.2 0.0 1.6 1.4 0.7 Overflow Queue (Q3), veh/ln 0.0 0.0 0.0 0.0 0.0 0.0 0.0 0.0 0.0 0.0 Queue Storage Ratio (RQ) (50th percentile) 0.21 0.10 0.10 0.02 0.02 0.01 0.01 0.20 0.12 0.06 32.6 36.5 Uniform Delay (d1), s/veh 4.8 3.6 3.6 4.7 3.5 3.1 32.7 31.8 Incremental Delay (d2), s/veh 0.5 0.4 0.4 0.2 0.3 0.1 0.0 0.4 0.4 0.2 Initial Queue Delay (d3), s/veh 0.0 0.0 0.0 0.0 0.0 0.0 0.0 0.0 0.0 0.0 Control Delay (d), s/veh 5.3 4.1 4.1 4.8 3.8 3.2 32.7 33.1 36.9 32.0 Level of Service (LOS) Α Α Α Α Α Α С С D С 4.3 Α 3.8 Α 33.1 С 35.2 Approach Delay, s/veh / LOS D Intersection Delay, s/veh / LOS 8.6 Α **Multimodal Results** ΕB WB NB SB Pedestrian LOS Score / LOS Bicycle LOS Score / LOS

HCS 2010 Signalized Intersection Intermediate Values 1474174 **General Information Intersection Information** Agency Infrastructure Engineering, Inc. Duration, h 0.25 Analyst Jane Canada Analysis Date 1/11/2016 Area Type Other PHF 1.00 Jurisdiction Time Period Intersection SR 46 & Westwood Boulev Analysis Year 2016 **Analysis Period** 1> 7:00 AM Existing 2016 - Ricker's @ SR 46 & Westwood Boulevard - Columbus, IN.xus File Name **Project Description** WB **Demand Information** EB NB SB Approach Movement R L R R R 682 58 Demand (v), veh/h 167 5 31 252 2 9 79 75 4 36 **Signal Information** د لله Cycle, s 82.0 Reference Phase 2 517 Offset, s 0 Reference Point Begin Green 10.8 60.0 0.0 0.0 0.0 0.0 Uncoordinated Yes Simult. Gap E/W On Yellow 3.9 4.3 0.0 0.0 0.0 0.0 Force Mode Fixed Simult. Gap N/S Red 1.5 1.5 0.0 0.0 0.0 0.0 FB WB NB SB Saturation Flow / Delay R L Τ R L R L R L Т Τ Τ Lane Width Adjustment Factor (fw) 1.000 1.000 1.000 1.000 1.000 1.000 1.000 1.000 1.000 1.000 1.000 1.000 Heavy Vehicle Adjustment Factor (fHV) 1.000 0.990 1.000 0.980 0.943 0.990 1.000 1.000 1.000 1.000 0.855 1.000 Approach Grade Adjustment Factor (fg) 1.000 1.000 1.000 1.000 1.000 1.000 1.000 1.000 1.000 1.000 1.000 1.000 Parking Activity Adjustment Factor (f_p) 1.000 1.000 1.000 1.000 1.000 1.000 1.000 1.000 1.000 1.000 1.000 1.000 Bus Blockage Adjustment Factor (fbb) 1.000 1.000 1.000 1.000 1.000 1.000 1.000 1.000 1.000 1.000 1.000 1.000 Area Type Adjustment Factor (fa) 1.000 1.000 1.000 1.000 1.000 1.000 1.000 1.000 1.000 1.000 1.000 1.000 Lane Utilization Adjustment Factor (fLU) 1.000 1.000 1.000 1.000 1.000 1.000 1.000 1.000 1.000 1.000 1.000 1.000 0.000 0.000 0.000 0.000 Left-Turn Adjustment Factor (f_LT) Right-Turn Adjustment Factor (fRT) 0.997 0.000 0.861 0.861 1.000 Left-Turn Pedestrian Adjustment Factor (fLpb) 1.000 1.000 1.000 Right-Turn Ped-Bike Adjustment Factor (*f*_{Rpb}) 1.000 1.000 1.000 1.000 Movement Saturation Flow Rate (s), veh/h 3730 1792 167 140 Platoon Ratio (R_p) 1.00 1.00 1.00 1.00 Proportion of Vehicles Arriving on Green (P) Incremental Delay Factor (k) 0.04 0.04 0.04 0.50 0.50 0.50 0.50 0.50 0.50 0.04 **Signal Timing / Movement Groups EBL** EBT/R **WBL** WBT/R **NBL** NBT/R SBL SBT/R Lost Time (t_L) 5.8 5.8 5.4 5.4 Green Ratio (g/C) 0.73 0.73 0.13 0.13 Permitted Saturation Flow Rate (sp), veh/h/ln 1086 752 1330 1389 Shared Saturation Flow Rate (ssh), veh/h/ln 60.0 60.0 10.9 10.9 Permitted Effective Green Time (g_p) , s Permitted Service Time (gu), s 56.4 55.0 8.9 6.9 Permitted Queue Service Time (qps), s 4.7 1.2 0.1 4.5 0.0 0.0 0.0 Time to First Blockage (gf), s 0.0 Queue Service Time Before Blockage (gfs), s Protected Right Saturation Flow (s_R), veh/h/ln 0 Protected Right Effective Green Time (g_R) , s 0.0 Multimodal FB **WB** NB SB Pedestrian Fw / Fv Pedestrian Fs / Fdelay Pedestrian Mcorner / Mcw Bicycle c b / d b

Bicycle Fw / Fv

No errors or warnings exist.

--- Comments ---

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HCS 2010™ Streets Version 6.41

Generated: 2/5/2016 9:20:58 AM

HCS 2010 Signalized Intersection Input Data 14. **General Information Intersection Information** Agency Infrastructure Engineering, Inc. Duration, h 0.25 Analyst Jane Canada Analysis Date 1/11/2016 Area Type Other PHF 1.00 Jurisdiction Time Period Intersection SR 46 & Westwood Boulev Analysis Year 2016 **Analysis Period** 1> 7:00 AM Proposed 2016 - Ricker's @ SR 46 & Westwood Boulevard - Columbus, IN.xus File Name **Project Description** EΒ WB **Demand Information** NB SB Approach Movement L R L R L R R 167 682 58 95 Demand (v), veh/h 7 45 252 3 10 75 6 36 **Signal Information** ٨, Cycle, s 83.0 Reference Phase 2 Offset, s 0 Reference Point Begin 60.0 0.0 0.0 0.0 Green 11.8 0.0 Uncoordinated Yes Simult. Gap E/W On Yellow 3.9 4.3 0.0 0.0 0.0 0.0 Force Mode Fixed Simult. Gap N/S On Red 1.5 1.5 0.0 0.0 0.0 0.0 **Traffic Information** EΒ WB NB SB Approach Movement R Т R L Τ L R L Τ L R Demand (v), veh/h 167 682 7 45 252 58 10 95 75 36 Initial Queue (Qb), veh/h 0 0 0 0 0 0 0 0 0 0 0 0 Base Saturation Flow Rate (so), veh/h 1900 1900 1900 1900 1900 1900 1900 1900 1900 1900 1900 1900 Parking (Nm), man/h None None None None Heavy Vehicles (PHV), % 0 1 2 6 1 0 0 0 17 Ped / Bike / RTOR, /h 0 0 0 0 0 0 0 0 0 0 0 0 Buses (Nb), buses/h 0 0 0 0 0 0 0 0 0 0 0 0 3 3 3 3 3 3 3 3 3 3 3 3 Arrival Type (AT) 1.00 1.00 1.00 Upstream Filtering (I) 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 Lane Width (W), ft 12.0 12.0 12.0 12.0 12.0 12.0 12.0 12.0 12.0 100 310 200 1150 750 160 195 290 Turn Bay Length, ft 313 Grade (P_g) , % 0 0 0 0 0 0 0 0 0 0 0 0 Speed Limit, mi/h 45 45 45 45 45 45 35 35 35 40 40 40 **Phase Information** EBT WBL WBT NBT SBL SBT **EBL** NBL Maximum Green (G_{max}) or Phase Split, s 60.0 60.0 30.0 30.0 4.3 Yellow Change Interval (Y), s 4.0 4.0 4.3 4.0 3.9 4.0 3.9 Red Clearance Interval (Rc), s 1.0 1.5 1.0 1.5 1.0 1.5 1.0 1.5 Minimum Green (Gmin), s 5 10 5 10 5 7 5 7 Start-Up Lost Time (It), s 2.0 2.0 2.0 2.0 2.0 2.0 2.0 2.0 Extension of Effective Green (e), s 2.0 2.0 2.0 2.0 2.0 2.0 2.0 2.0 Passage (PT), s 2.0 3.0 2.0 3.0 2.0 1.5 2.0 1.5 Recall Mode Min Max Min Max Min Min Min Min **Dual Entry** No Yes No Yes No Yes Nο Yes 0.0 Walk (Walk), s 0.0 0.0 0.0 0.0 0.0 0.0 0.0 Pedestrian Clearance Time (PC), s 0.0 0.0 0.0 0.0 0.0 0.0 0.0 0.0 **Multimodal Information** EΒ WB NB SB 85th % Speed / Rest in Walk / Corner Radius 25 0 25 0 25 0 25 0 No No No Nο 9.0 0 9.0 0 9.0 12 0 9.0 12 0 Walkway / Crosswalk Width / Length, ft 12 12 Street Width / Island / Curb 0 0 0 0 0 0 0 0 No No No No Width Outside / Bike Lane / Shoulder, ft 12 5.0 2.0 12 5.0 2.0 12 5.0 2.0 12 5.0 2.0

Pedestrian Signal / Occupied Parking

No

0.50

No

No

0.50

0.50

0.50

No

Generated: 2/5/2016 9:29:39 AM

HCS 2010 Signalized Intersection Results Summary Intersection Information 14. 4177 **General Information** Agency Infrastructure Engineering, Inc. Duration, h 0.25 Analyst Jane Canada Analysis Date 1/11/2016 Area Type Other PHF 1.00 Jurisdiction Time Period Intersection SR 46 & Westwood Boulev Analysis Year 2016 **Analysis Period** 1> 7:00 AM Proposed 2016 - Ricker's @ SR 46 & Westwood Boulevard - Columbus, IN.xus File Name **Project Description** WB **Demand Information** EB NB SB Approach Movement R L R L R R 682 58 95 Demand (v), veh/h 167 7 45 252 3 10 75 6 36 **Signal Information** ؞؞ڶڶ؞ Cycle, s 83.0 Reference Phase 2 Offset, s 0 Reference Point Begin 60.0 0.0 0.0 0.0 Green 11.8 0.0 Uncoordinated Yes Simult. Gap E/W On Yellow 3.9 4.3 0.0 0.0 0.0 0.0 Force Mode Fixed Simult. Gap N/S 0.0 On Red 1.5 1.5 0.0 0.0 0.0 **Timer Results EBL EBT WBL** WBT NBL **NBT** SBL SBT **Assigned Phase** 4 8 2 6 Case Number 6.0 5.0 6.0 6.0 Phase Duration, s 65.8 65.8 17.2 17.2 Change Period, (Y+Rc), s 5.8 5.8 5.4 5.4 Max Allow Headway (MAH), s 4.1 4.1 2.8 2.8 Queue Clearance Time (gs), s 10.7 9.0 6.9 11.5 Green Extension Time (ge), s 5.1 5.2 0.3 0.3 Phase Call Probability 1.00 1.00 1.00 1.00 0.00 0.00 0.00 0.00 Max Out Probability WB NB SB **Movement Group Results** EΒ Approach Movement L Т R L Т R L Т R L Т R **Assigned Movement** 7 4 14 3 8 18 5 2 12 1 6 16 Adjusted Flow Rate (v), veh/h 167 345 344 45 252 58 3 105 75 42 1086 1881 1874 751 1792 1594 1386 1634 1309 1407 Adjusted Saturation Flow Rate (s), veh/h/ln 4.9 5.2 5.2 1.8 3.8 0.2 4.9 4.6 2.2 Queue Service Time (gs), s 0.9 Cycle Queue Clearance Time (gc), s 8.7 5.2 5.2 7.0 3.8 0.9 2.3 4.9 9.5 2.2 821 1358 1353 582 1294 250 234 201 1151 198 Capacity (c), veh/h Volume-to-Capacity Ratio (X) 0.203 0.254 0.254 0.077 0.195 0.050 0.012 0.449 0.379 0.208 Available Capacity (ca), veh/h 821 1358 1353 582 1294 1151 551 590 483 508 Back of Queue (Q), veh/ln (50th percentile) 0.9 1.4 1.4 0.2 1.0 0.2 0.1 1.9 1.4 0.7 Overflow Queue (Q3), veh/ln 0.0 0.0 0.0 0.0 0.0 0.0 0.0 0.0 0.0 0.0 Queue Storage Ratio (RQ) (50th percentile) 0.22 0.11 0.11 0.03 0.02 0.01 0.01 0.24 0.12 0.07 32.6 36.9 Uniform Delay (d1), s/veh 5.1 3.9 3.9 5.1 3.7 3.3 32.4 31.4 Incremental Delay (d2), s/veh 0.6 0.5 0.5 0.3 0.3 0.1 0.0 0.5 0.4 0.2 Initial Queue Delay (d3), s/veh 0.0 0.0 0.0 0.0 0.0 0.0 0.0 0.0 0.0 0.0 Control Delay (d), s/veh 5.7 4.4 4.4 5.4 4.1 3.4 32.4 33.1 37.3 31.6 Level of Service (LOS) Α Α Α Α Α Α С С D С 4.6 Α 4.1 Α 33.1 С 35.3 Approach Delay, s/veh / LOS D Intersection Delay, s/veh / LOS 9.2 Α **Multimodal Results** ΕB WB NB SB Pedestrian LOS Score / LOS Bicycle LOS Score / LOS

HCS 2010 Signalized Intersection Intermediate Values 1474174 **General Information Intersection Information** Agency Infrastructure Engineering, Inc. Duration, h 0.25 Analyst Jane Canada Analysis Date 1/11/2016 Area Type Other PHF 1.00 Jurisdiction Time Period Intersection SR 46 & Westwood Boulev Analysis Year 2016 **Analysis Period** 1> 7:00 AM Proposed 2016 - Ricker's @ SR 46 & Westwood Boulevard - Columbus, IN.xus File Name **Project Description** WB **Demand Information** EB NB SB Approach Movement R L R R R 682 45 58 95 Demand (v), veh/h 167 7 252 3 10 75 6 36 **Signal Information** ىل:، Cycle, s 83.0 Reference Phase 2 517 Offset, s 0 Reference Point Begin Green 11.8 60.0 0.0 0.0 0.0 0.0 Uncoordinated Yes Simult. Gap E/W On Yellow 3.9 4.3 0.0 0.0 0.0 0.0 Force Mode Fixed Simult. Gap N/S Red 1.5 1.5 0.0 0.0 0.0 0.0 FB WB NB SB Saturation Flow / Delay R L Τ R L R L R L Т Τ Τ Lane Width Adjustment Factor (fw) 1.000 1.000 1.000 1.000 1.000 1.000 1.000 1.000 1.000 1.000 1.000 1.000 Heavy Vehicle Adjustment Factor (fHV) 1.000 0.990 1.000 0.980 0.943 0.990 1.000 1.000 1.000 1.000 0.855 1.000 Approach Grade Adjustment Factor (fg) 1.000 1.000 1.000 1.000 1.000 1.000 1.000 1.000 1.000 1.000 1.000 1.000 Parking Activity Adjustment Factor (f_p) 1.000 1.000 1.000 1.000 1.000 1.000 1.000 1.000 1.000 1.000 1.000 1.000 Bus Blockage Adjustment Factor (fbb) 1.000 1.000 1.000 1.000 1.000 1.000 1.000 1.000 1.000 1.000 1.000 1.000 Area Type Adjustment Factor (fa) 1.000 1.000 1.000 1.000 1.000 1.000 1.000 1.000 1.000 1.000 1.000 1.000 Lane Utilization Adjustment Factor (fLU) 1.000 1.000 1.000 1.000 1.000 1.000 1.000 1.000 1.000 1.000 1.000 1.000 0.000 0.000 0.000 0.000 Left-Turn Adjustment Factor (f_LT) Right-Turn Adjustment Factor (fRT) 0.996 0.000 0.860 0.866 1.000 Left-Turn Pedestrian Adjustment Factor (fLpb) 1.000 1.000 1.000 Right-Turn Ped-Bike Adjustment Factor (*f*_{Rpb}) 1.000 1.000 1.000 1.000 Movement Saturation Flow Rate (s), veh/h 3717 1792 156 201 Platoon Ratio (R_p) 1.00 1.00 1.00 1.00 Proportion of Vehicles Arriving on Green (P) Incremental Delay Factor (k) 0.04 0.04 0.04 0.50 0.50 0.50 0.50 0.50 0.50 0.04 WBT/R **Signal Timing / Movement Groups EBL** EBT/R **WBL NBL** NBT/R SBL SBT/R Lost Time (t_L) 5.8 5.8 5.4 5.4 Green Ratio (g/C) 0.72 0.72 0.14 0.14 Permitted Saturation Flow Rate (sp), veh/h/ln 1086 1309 751 1386 Shared Saturation Flow Rate (ssh), veh/h/ln 60.0 60.0 11.9 11.9 Permitted Effective Green Time (g_p) , s Permitted Service Time (gu), s 56.2 54.8 9.8 7.1 Permitted Queue Service Time (qps), s 4.6 4.9 1.8 0.2 0.0 0.0 0.0 Time to First Blockage (gf), s 0.0 Queue Service Time Before Blockage (gfs), s Protected Right Saturation Flow (s_R), veh/h/ln 0 Protected Right Effective Green Time (g_R) , s 0.0 Multimodal FB **WB** NB SB Pedestrian Fw / Fv Pedestrian Fs / Fdelay Pedestrian Mcorner / Mcw

Bicycle c b / d b
Bicycle Fw / Fv

No errors or warnings exist.

--- Comments ---

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HCS 2010 Signalized Intersection Input Data 14. **General Information Intersection Information** Agency Infrastructure Engineering, Inc. Duration, h 0.25 Analyst Jane Canada Analysis Date 1/11/2016 Area Type Other PHF 1.00 Jurisdiction Time Period Intersection SR 46 & Westwood Boulev Analysis Year 2016 **Analysis Period** 1> 7:00 PM Existing 2016 - Ricker's @ SR 46 & Westwood Boulevard - Columbus, IN.xus File Name **Project Description** EΒ WB **Demand Information** NB SB Approach Movement L R L R L R L R 405 19 52 43 Demand (v), veh/h 77 97 556 12 10 150 39 179 **Signal Information** ؞؞ڶڶ؞ Cycle, s 87.7 Reference Phase 2 Offset, s 0 Reference Point Begin Green 16.5 60.0 0.0 0.0 0.0 0.0 Uncoordinated Yes Simult. Gap E/W On Yellow 3.9 4.3 0.0 0.0 0.0 0.0 Force Mode Fixed Simult. Gap N/S On Red 1.5 1.5 0.0 0.0 0.0 0.0 **Traffic Information** EΒ WB NB SB Approach Movement R Τ R R L Τ L L Τ L Т R Demand (v), veh/h 77 405 19 556 52 12 10 43 150 39 179 Initial Queue (Qb), veh/h 0 0 0 0 0 0 0 0 0 0 0 0 Base Saturation Flow Rate (so), veh/h 1900 1900 1900 1900 1900 1900 1900 1900 1900 1900 1900 1900 Parking (Nm), man/h None None None None Heavy Vehicles (PHV), % 0 1 2 6 1 0 0 0 17 Ped / Bike / RTOR, /h 0 0 0 0 0 0 0 0 0 0 0 0 Buses (Nb), buses/h 0 0 0 0 0 0 0 0 0 0 0 0 3 3 3 3 3 3 3 3 3 3 3 3 Arrival Type (AT) 1.00 1.00 1.00 Upstream Filtering (I) 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 Lane Width (W), ft 12.0 12.0 12.0 12.0 12.0 12.0 12.0 12.0 12.0 100 310 200 1150 750 160 195 290 Turn Bay Length, ft 313 Grade (P_g) , % 0 0 0 0 0 0 0 0 0 0 0 0 Speed Limit, mi/h 45 45 45 45 45 45 35 35 35 35 35 35 **Phase Information** EBT WBL WBT NBT SBL SBT **EBL** NBL Maximum Green (G_{max}) or Phase Split, s 60.0 60.0 35.0 35.0 4.3 Yellow Change Interval (Y), s 4.0 4.0 4.3 4.0 3.9 4.0 3.9 Red Clearance Interval (Rc), s 1.0 1.5 1.0 1.5 1.0 1.5 1.0 1.5 Minimum Green (Gmin), s 5 10 5 10 5 7 5 7 Start-Up Lost Time (It), s 2.0 2.0 2.0 2.0 2.0 2.0 2.0 2.0 Extension of Effective Green (e), s 2.0 2.0 2.0 2.0 2.0 2.0 2.0 2.0 Passage (PT), s 2.0 3.0 2.0 3.0 2.0 1.5 2.0 1.5 Recall Mode Min Max Min Max Min Min Min Min **Dual Entry** No Yes No Yes No Yes Nο Yes 0.0 Walk (Walk), s 0.0 0.0 0.0 0.0 0.0 0.0 0.0 Pedestrian Clearance Time (PC), s 0.0 0.0 0.0 0.0 0.0 0.0 0.0 0.0 **Multimodal Information** EΒ WB NB SB 85th % Speed / Rest in Walk / Corner Radius 25 0 25 0 25 0 25 0 No No No Nο 9.0 0 9.0 0 9.0 12 0 9.0 12 0 Walkway / Crosswalk Width / Length, ft 12 12 Street Width / Island / Curb 0 0 0 0 0 0 0 0 No No No No Width Outside / Bike Lane / Shoulder, ft 12 5.0 2.0 12 5.0 2.0 12 5.0 2.0 12 5.0 2.0

Pedestrian Signal / Occupied Parking

No

0.50

No

No

0.50

0.50

0.50

No

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HCS 2010 Signalized Intersection Results Summary Intersection Information 14. 4177 **General Information** Agency Infrastructure Engineering, Inc. Duration, h 0.25 Analyst Jane Canada Analysis Date 1/11/2016 Area Type Other PHF 1.00 Jurisdiction Time Period Intersection SR 46 & Westwood Boulev Analysis Year 2016 **Analysis Period** 1> 7:00 PM Existing 2016 - Ricker's @ SR 46 & Westwood Boulevard - Columbus, IN.xus File Name **Project Description** EΒ WB **Demand Information** NB SB Approach Movement L R L R L R L R 405 52 43 Demand (v), veh/h 77 19 97 556 12 10 150 39 179 **Signal Information** ؞؞ڶڶ؞ Cycle, s 87.7 Reference Phase 2 Offset, s 0 Reference Point Begin 60.0 0.0 0.0 0.0 Green 16.5 0.0 Uncoordinated Yes Simult. Gap E/W On Yellow 3.9 4.3 0.0 0.0 0.0 0.0 Force Mode Fixed Simult. Gap N/S 0.0 On Red 1.5 1.5 0.0 0.0 0.0 **Timer Results EBL EBT WBL** WBT NBL **NBT** SBL SBT **Assigned Phase** 4 8 2 6 Case Number 6.0 5.0 6.0 6.0 Phase Duration, s 65.8 65.8 21.9 21.9 Change Period, (Y+Rc), s 5.8 5.8 5.4 5.4 Max Allow Headway (MAH), s 4.1 4.1 2.8 2.8 Queue Clearance Time (gs), s 18.5 14.4 15.9 15.0 Green Extension Time (ge), s 5.3 5.4 0.6 0.6 Phase Call Probability 1.00 1.00 1.00 1.00 0.00 0.00 0.00 0.00 Max Out Probability WB NB SB **Movement Group Results** EΒ Approach Movement L Т R L Т R L Т R L Т R **Assigned Movement** 7 4 14 3 8 18 5 2 12 1 6 16 Adjusted Flow Rate (v), veh/h 77 213 211 97 556 52 12 53 150 218 826 1881 1851 959 1792 1594 1182 1658 1373 1415 Adjusted Saturation Flow Rate (s), veh/h/ln 4.1 3.5 3.6 3.5 12.4 0.9 2.4 9.0 13.0 Queue Service Time (gs), s 0.9 Cycle Queue Clearance Time (gc), s 16.5 3.5 3.6 7.1 12.4 0.9 13.9 2.4 11.4 13.0 530 1267 700 1227 303 266 1287 1091 129 311 Capacity (c), veh/h Volume-to-Capacity Ratio (X) 0.145 0.166 0.166 0.139 0.453 0.048 0.093 0.170 0.496 0.820 Available Capacity (ca), veh/h 530 1287 1267 700 1227 1091 379 662 593 565 Back of Queue (Q), veh/ln (50th percentile) 0.7 0.6 0.2 0.3 0.9 3.0 4.5 1.1 1.1 3.7 Overflow Queue (Q3), veh/ln 0.0 0.0 0.0 0.0 0.0 0.0 0.0 0.0 0.0 0.0 Queue Storage Ratio (RQ) (50th percentile) 0.18 0.09 0.09 0.08 0.09 0.01 0.04 0.12 0.25 0.41 29.9 34.7 Uniform Delay (d1), s/veh 10.1 4.9 4.9 6.2 6.3 4.5 40.9 34.2 Incremental Delay (d2), s/veh 0.6 0.3 0.3 0.4 1.2 0.1 0.1 0.1 0.5 2.4 Initial Queue Delay (d3), s/veh 0.0 0.0 0.0 0.0 0.0 0.0 0.0 0.0 0.0 0.0 Control Delay (d), s/veh 10.7 5.2 5.2 6.6 7.5 4.6 41.0 30.0 35.1 36.6 Level of Service (LOS) В Α Α Α Α Α D С D D 6.0 Α 7.2 Α 32.0 С 36.0 Approach Delay, s/veh / LOS D Intersection Delay, s/veh / LOS В 14.3 **Multimodal Results** ΕB WB NB SB Pedestrian LOS Score / LOS Bicycle LOS Score / LOS

HCS 2010 Signalized Intersection Intermediate Values 1474174 **General Information Intersection Information** Agency Infrastructure Engineering, Inc. Duration, h 0.25 Analyst Jane Canada Analysis Date 1/11/2016 Area Type Other PHF 1.00 Jurisdiction Time Period Intersection SR 46 & Westwood Boulev Analysis Year 2016 **Analysis Period** 1> 7:00 PM Existing 2016 - Ricker's @ SR 46 & Westwood Boulevard - Columbus, IN.xus File Name **Project Description** WB **Demand Information** EB NB SB Approach Movement R L R L R L R 405 52 43 Demand (v), veh/h 77 19 97 556 12 10 150 39 179 **Signal Information** د لله Cycle, s 87.7 Reference Phase 2 Offset, s 0 Reference Point Begin Green 16.5 60.0 0.0 0.0 0.0 0.0 Uncoordinated Yes Simult. Gap E/W On Yellow 3.9 4.3 0.0 0.0 0.0 0.0 Force Mode Fixed Simult. Gap N/S Red 1.5 1.5 0.0 0.0 0.0 0.0 FB WB NB SB Saturation Flow / Delay R L Τ R L R L R L Т Τ Τ Lane Width Adjustment Factor (fw) 1.000 1.000 1.000 1.000 1.000 1.000 1.000 1.000 1.000 1.000 1.000 1.000 Heavy Vehicle Adjustment Factor (fHV) 1.000 0.990 1.000 0.980 0.943 0.990 1.000 1.000 1.000 1.000 0.855 1.000 Approach Grade Adjustment Factor (fg) 1.000 1.000 1.000 1.000 1.000 1.000 1.000 1.000 1.000 1.000 1.000 1.000 Parking Activity Adjustment Factor (f_p) 1.000 1.000 1.000 1.000 1.000 1.000 1.000 1.000 1.000 1.000 1.000 1.000 Bus Blockage Adjustment Factor (fbb) 1.000 1.000 1.000 1.000 1.000 1.000 1.000 1.000 1.000 1.000 1.000 1.000 Area Type Adjustment Factor (fa) 1.000 1.000 1.000 1.000 1.000 1.000 1.000 1.000 1.000 1.000 1.000 1.000 Lane Utilization Adjustment Factor (fLU) 1.000 1.000 1.000 1.000 1.000 1.000 1.000 1.000 1.000 1.000 1.000 1.000 0.000 0.000 0.000 0.000 Left-Turn Adjustment Factor (f_LT) Right-Turn Adjustment Factor (fRT) 0.984 0.000 0.873 0.871 1.000 Left-Turn Pedestrian Adjustment Factor (fLpb) 1.000 1.000 1.000 Right-Turn Ped-Bike Adjustment Factor (*f*_{Rpb}) 1.000 1.000 1.000 1.000 Movement Saturation Flow Rate (s), veh/h 3567 1792 313 253 Platoon Ratio (R_p) 1.00 1.00 1.00 1.00 Proportion of Vehicles Arriving on Green (P) Incremental Delay Factor (k) 0.04 0.04 0.04 0.50 0.50 0.50 0.50 0.50 0.50 0.04 **Signal Timing / Movement Groups EBL** EBT/R **WBL** WBT/R **NBL** NBT/R SBL SBT/R Lost Time (t_L) 5.8 5.8 5.4 5.4 Green Ratio (g/C) 0.68 0.68 0.19 0.19 Permitted Saturation Flow Rate (sp), veh/h/ln 826 959 1182 1373 Shared Saturation Flow Rate (ssh), veh/h/ln 60.0 60.0 16.5 16.5 Permitted Effective Green Time (g_p) , s Permitted Service Time (gu), s 47.6 56.5 3.5 14.1 Permitted Queue Service Time (qps), s 4.1 9.0 3.5 0.9 0.0 0.0 0.0 Time to First Blockage (gf), s 0.0 Queue Service Time Before Blockage (gfs), s Protected Right Saturation Flow (s_R), veh/h/ln 0 Protected Right Effective Green Time (g_R) , s 0.0 Multimodal FB **WB** NB SB Pedestrian Fw / Fv Pedestrian Fs / Fdelay Pedestrian Mcorner / Mcw

Bicycle c b / d b
Bicycle Fw / Fv

No errors or warnings exist.

--- Comments ---

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HCS 2010 Signalized Intersection Input Data 14. **General Information Intersection Information** Agency Infrastructure Engineering, Inc. Duration, h 0.25 Analyst Jane Canada Analysis Date 1/11/2016 Area Type Other PHF 1.00 Jurisdiction Time Period Intersection SR 46 & Westwood Boulev Analysis Year 2016 **Analysis Period** 1> 7:00 PM Proposed 2016 - Ricker's @ SR 46 & Westwood Boulevard - Columbus, IN.xus File Name **Project Description** WB **Demand Information** EB NB SB Approach Movement L R L R L R L R 405 24 52 68 Demand (v), veh/h 77 118 556 19 11 150 48 179 **Signal Information** ؞؞ڶڶ؞ Cycle, s 88.8 Reference Phase 2 Offset, s 0 Reference Point Begin Green 17.6 60.0 0.0 0.0 0.0 0.0 Uncoordinated Yes Simult. Gap E/W On Yellow 3.9 4.3 0.0 0.0 0.0 0.0 Force Mode Fixed Simult. Gap N/S On Red 1.5 1.5 0.0 0.0 0.0 0.0 **Traffic Information** EΒ WB NB SB Approach Movement R Τ R R L Τ L L Τ L Т R Demand (v), veh/h 77 405 24 118 556 52 19 11 68 150 48 179 Initial Queue (Qb), veh/h 0 0 0 0 0 0 0 0 0 0 0 0 Base Saturation Flow Rate (so), veh/h 1900 1900 1900 1900 1900 1900 1900 1900 1900 1900 1900 1900 Parking (Nm), man/h None None None None Heavy Vehicles (PHV), % 0 1 2 6 1 0 0 0 17 Ped / Bike / RTOR, /h 0 0 0 0 0 0 0 0 0 0 0 0 Buses (Nb), buses/h 0 0 0 0 0 0 0 0 0 0 0 0 3 3 3 3 3 3 3 3 3 3 3 3 Arrival Type (AT) 1.00 1.00 1.00 Upstream Filtering (I) 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 Lane Width (W), ft 12.0 12.0 12.0 12.0 12.0 12.0 12.0 12.0 12.0 100 310 200 1150 750 160 195 290 Turn Bay Length, ft 313 Grade (P_g) , % 0 0 0 0 0 0 0 0 0 0 0 0 Speed Limit, mi/h 45 45 45 45 45 45 35 35 35 40 40 40 **Phase Information** EBT WBL WBT NBT SBL SBT **EBL NBL** Maximum Green (G_{max}) or Phase Split, s 60.0 60.0 35.0 35.0 4.3 Yellow Change Interval (Y), s 4.0 4.0 4.3 4.0 3.9 4.0 3.9 Red Clearance Interval (Rc), s 1.0 1.5 1.0 1.5 1.0 1.5 1.0 1.5 Minimum Green (Gmin), s 5 10 5 10 5 7 5 7 Start-Up Lost Time (It), s 2.0 2.0 2.0 2.0 2.0 2.0 2.0 2.0 Extension of Effective Green (e), s 2.0 2.0 2.0 2.0 2.0 2.0 2.0 2.0 Passage (PT), s 2.0 3.0 2.0 3.0 2.0 1.5 2.0 1.5 Recall Mode Min Max Min Off Min Min Min Min **Dual Entry** No Yes No Yes No Yes Nο Yes 0.0 Walk (Walk), s 0.0 0.0 0.0 0.0 0.0 0.0 0.0 Pedestrian Clearance Time (PC), s 0.0 0.0 0.0 0.0 0.0 0.0 0.0 0.0 **Multimodal Information** EΒ WB NB SB 85th % Speed / Rest in Walk / Corner Radius 25 0 25 0 25 0 25 0 No No No Nο 9.0 0 9.0 0 9.0 12 0 9.0 12 0 Walkway / Crosswalk Width / Length, ft 12 12 Street Width / Island / Curb 0 0 0 0 0 0 0 0 No No No No Width Outside / Bike Lane / Shoulder, ft 12 5.0 2.0 12 5.0 2.0 12 5.0 2.0 12 5.0 2.0

Pedestrian Signal / Occupied Parking

No

0.50

No

No

0.50

0.50

0.50

HCS 2010 Signalized Intersection Results Summary Intersection Information 14. 4177 **General Information** Agency Infrastructure Engineering, Inc. Duration, h 0.25 Analyst Jane Canada Analysis Date 1/11/2016 Area Type Other PHF 1.00 Jurisdiction Time Period Intersection SR 46 & Westwood Boulev Analysis Year 2016 Analysis Period 1> 7:00 PM Proposed 2016 - Ricker's @ SR 46 & Westwood Boulevard - Columbus, IN.xus File Name **Project Description** WB **Demand Information** EB NB SB Approach Movement L R L R L R L R 405 24 556 52 68 Demand (v), veh/h 77 118 19 11 150 48 179 **Signal Information** ؞؞ڶڶ؞ Cycle, s 8.88 Reference Phase 2 Offset, s 0 Reference Point Begin Green 17.6 60.0 0.0 0.0 0.0 0.0 Uncoordinated Yes Simult. Gap E/W On Yellow 3.9 4.3 0.0 0.0 0.0 0.0 Force Mode Fixed Simult. Gap N/S 0.0 On Red 1.5 1.5 0.0 0.0 0.0 **Timer Results EBL EBT WBL** WBT NBL **NBT** SBL SBT **Assigned Phase** 4 8 2 6 Case Number 6.0 5.0 6.0 6.0 Phase Duration, s 65.8 65.8 23.0 23.0 Change Period, (Y+Rc), s 5.8 5.8 5.4 5.4 Max Allow Headway (MAH), s 4.2 4.2 2.7 2.7 Queue Clearance Time (gs), s 19.2 15.0 16.9 15.5 Green Extension Time (ge), s 5.5 5.6 0.6 0.6 Phase Call Probability 1.00 1.00 1.00 1.00 0.00 0.00 0.00 0.00 Max Out Probability WB NB SB **Movement Group Results** EΒ Approach Movement L Т R L Т R L Т R L Т R **Assigned Movement** 7 4 14 3 8 18 5 2 12 1 6 16 Adjusted Flow Rate (v), veh/h 77 216 213 118 556 52 19 79 150 227 Adjusted Saturation Flow Rate (s), veh/h/ln 826 1881 1844 955 1792 1594 1172 1645 1341 1422 4.3 3.7 3.8 13.0 1.4 3.6 9.4 13.5 Queue Service Time (gs), s 4.6 1.0 17.2 Cycle Queue Clearance Time (gc), s 3.7 3.8 8.4 13.0 1.0 14.9 3.6 13.0 13.5 518 1211 326 293 282 1271 1245 686 1077 135 Capacity (c), veh/h Volume-to-Capacity Ratio (X) 0.149 0.170 0.171 0.172 0.459 0.048 0.141 0.242 0.513 0.804 Available Capacity (ca), veh/h 518 1271 1245 686 1211 1077 364 648 555 560 Back of Queue (Q), veh/ln (50th percentile) 8.0 1.2 1.2 8.0 3.7 0.2 0.4 1.4 3.0 4.6 Overflow Queue (Q3), veh/ln 0.0 0.0 0.0 0.0 0.0 0.0 0.0 0.0 0.0 0.0 Queue Storage Ratio (RQ) (50th percentile) 0.19 0.10 0.10 0.10 0.08 0.01 0.06 0.18 0.26 0.42 30.0 35.5 Uniform Delay (d1), s/veh 10.8 5.3 5.3 6.8 6.8 4.8 41.1 34.0 Incremental Delay (d2), s/veh 0.6 0.3 0.3 0.1 0.3 0.0 0.2 0.1 0.5 2.1 Initial Queue Delay (d3), s/veh 0.0 0.0 0.0 0.0 0.0 0.0 0.0 0.0 0.0 0.0 Control Delay (d), s/veh 11.4 5.6 5.6 6.9 7.1 4.9 41.3 30.1 36.0 36.0 Level of Service (LOS) В Α Α Α Α Α D С D D 6.5 Α 6.9 Α 32.3 С 36.0 Approach Delay, s/veh / LOS D Intersection Delay, s/veh / LOS В 14.6 **Multimodal Results** ΕB WB NB SB Pedestrian LOS Score / LOS Bicycle LOS Score / LOS

HCS 2010 Signalized Intersection Intermediate Values 1474174 **General Information Intersection Information** Agency Infrastructure Engineering, Inc. Duration, h 0.25 Analyst Jane Canada Analysis Date 1/11/2016 Area Type Other PHF 1.00 Jurisdiction Time Period Intersection SR 46 & Westwood Boulev Analysis Year 2016 **Analysis Period** 1> 7:00 PM Proposed 2016 - Ricker's @ SR 46 & Westwood Boulevard - Columbus, IN.xus File Name **Project Description** WB **Demand Information** EB NB SB Approach Movement R L R L R L R 405 24 52 68 Demand (v), veh/h 77 118 556 19 11 150 48 179 **Signal Information** ىل:، Cycle, s 8.88 Reference Phase 2 517 Offset, s 0 Reference Point Begin Green 17.6 60.0 0.0 0.0 0.0 0.0 Uncoordinated Yes Simult. Gap E/W On Yellow 3.9 4.3 0.0 0.0 0.0 0.0 Force Mode Fixed Simult. Gap N/S Red 1.5 1.5 0.0 0.0 0.0 0.0 FB WB NB SB Saturation Flow / Delay R L Τ R L R L R L Т Τ Τ Lane Width Adjustment Factor (fw) 1.000 1.000 1.000 1.000 1.000 1.000 1.000 1.000 1.000 1.000 1.000 1.000 Heavy Vehicle Adjustment Factor (fHV) 1.000 0.990 1.000 0.980 0.943 0.990 1.000 1.000 1.000 1.000 0.855 1.000 Approach Grade Adjustment Factor (fg) 1.000 1.000 1.000 1.000 1.000 1.000 1.000 1.000 1.000 1.000 1.000 1.000 Parking Activity Adjustment Factor (f_p) 1.000 1.000 1.000 1.000 1.000 1.000 1.000 1.000 1.000 1.000 1.000 1.000 Bus Blockage Adjustment Factor (fbb) 1.000 1.000 1.000 1.000 1.000 1.000 1.000 1.000 1.000 1.000 1.000 1.000 Area Type Adjustment Factor (fa) 1.000 1.000 1.000 1.000 1.000 1.000 1.000 1.000 1.000 1.000 1.000 1.000 Lane Utilization Adjustment Factor (fLU) 1.000 1.000 1.000 1.000 1.000 1.000 1.000 1.000 1.000 1.000 1.000 1.000 0.000 0.000 0.000 0.000 Left-Turn Adjustment Factor (f_LT) 0.980 0.000 0.866 0.876 Right-Turn Adjustment Factor (frt) 1.000 Left-Turn Pedestrian Adjustment Factor (fLpb) 1.000 1.000 1.000 Right-Turn Ped-Bike Adjustment Factor (*f*_{Rpb}) 1.000 1.000 1.000 1.000 229 Movement Saturation Flow Rate (s), veh/h 3519 1792 301 Platoon Ratio (R_p) 1.00 1.00 1.00 1.00 Proportion of Vehicles Arriving on Green (P) Incremental Delay Factor (k) 0.04 0.04 0.04 0.50 0.50 0.50 0.11 0.11 0.11 0.04 **Signal Timing / Movement Groups EBL** EBT/R **WBL** WBT/R **NBL** NBT/R SBL SBT/R Lost Time (t_L) 5.8 5.8 5.4 5.4 Green Ratio (g/C) 0.68 0.68 0.20 0.20 Permitted Saturation Flow Rate (sp), veh/h/ln 1172 1341 826 955 Shared Saturation Flow Rate (ssh), veh/h/ln 60.0 60.0 17.6 17.6 Permitted Effective Green Time (g_p) , s Permitted Service Time (gu), s 47.1 56.2 4.1 14.0 Permitted Queue Service Time (qps), s 4.3 9.4 4.6 1.4 0.0 0.0 0.0 Time to First Blockage (gf), s 0.0 Queue Service Time Before Blockage (gfs), s Protected Right Saturation Flow (s_R), veh/h/ln 0 Protected Right Effective Green Time (g_R) , s 0.0 Multimodal FB **WB** NB SB Pedestrian Fw / Fv Pedestrian Fs / Fdelay Pedestrian Mcorner / Mcw

Bicycle c b / d b
Bicycle Fw / Fv

No errors or warnings exist.

--- Comments ---

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Appendix E – HCS 2010 Capacity Analysis for Year 2036

HCS 2010 Signalized Intersection Input Data 14. **General Information Intersection Information** Agency Infrastructure Engineering, Inc. Duration, h 0.25 Analyst Jane Canada Analysis Date 1/11/2016 Area Type Other PHF 1.00 Jurisdiction Time Period Intersection SR 46 & Westwood Boulev Analysis Year 2016 **Analysis Period** 1> 7:00 AM Existing 2036 - Ricker's @ SR 46 & Westwood Boulevard - Columbus, IN.xus File Name **Project Description** WB **Demand Information** EB NB SB Approach Movement R L R L R R 203 833 308 97 5 Demand (v), veh/h 6 37 71 2 11 91 44 **Signal Information** ٨, Cycle, s 84.3 Reference Phase 2 Offset, s 0 Reference Point Begin Green 13.1 60.0 0.0 0.0 0.0 0.0 Uncoordinated Yes Simult. Gap E/W On Yellow 3.9 4.3 0.0 0.0 0.0 0.0 Force Mode Fixed Simult. Gap N/S On Red 1.5 1.5 0.0 0.0 0.0 0.0 **Traffic Information** EΒ WB NB SB Approach Movement R Τ R L Τ L L Τ R L R Demand (v), veh/h 203 833 6 37 308 71 11 97 91 44 Initial Queue (Qb), veh/h 0 0 0 0 0 0 0 0 0 0 0 0 Base Saturation Flow Rate (so), veh/h 1900 1900 1900 1900 1900 1900 1900 1900 1900 1900 1900 1900 Parking (Nm), man/h None None None None Heavy Vehicles (PHV), % 0 1 2 6 1 0 0 0 17 Ped / Bike / RTOR, /h 0 0 0 0 0 0 0 0 0 0 0 0 Buses (Nb), buses/h 0 0 0 0 0 0 0 0 0 0 0 0 3 3 3 3 3 3 3 3 3 3 3 3 Arrival Type (AT) 1.00 1.00 1.00 Upstream Filtering (I) 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 Lane Width (W), ft 12.0 12.0 12.0 12.0 12.0 12.0 12.0 12.0 12.0 100 310 200 1150 750 160 195 290 Turn Bay Length, ft 313 Grade (P_g) , % 0 0 0 0 0 0 0 0 0 0 0 0 Speed Limit, mi/h 45 45 45 45 45 45 35 35 35 40 40 40 **Phase Information** WBL WBT NBT SBL SBT **EBL EBT NBL** Maximum Green (G_{max}) or Phase Split, s 60.0 60.0 30.0 30.0 4.3 Yellow Change Interval (Y), s 4.0 4.0 4.3 4.0 3.9 4.0 3.9 Red Clearance Interval (Rc), s 1.0 1.5 1.0 1.5 1.0 1.5 1.0 1.5 Minimum Green (Gmin), s 5 10 5 10 5 7 5 7 Start-Up Lost Time (It), s 2.0 2.0 2.0 2.0 2.0 2.0 2.0 2.0 Extension of Effective Green (e), s 2.0 2.0 2.0 2.0 2.0 2.0 2.0 2.0 Passage (PT), s 2.0 3.0 2.0 3.0 2.0 1.5 2.0 1.5 Recall Mode Min Max Min Max Min Min Min Min **Dual Entry** No Yes No Yes No Yes Nο Yes 0.0 Walk (Walk), s 0.0 0.0 0.0 0.0 0.0 0.0 0.0 Pedestrian Clearance Time (PC), s 0.0 0.0 0.0 0.0 0.0 0.0 0.0 0.0 **Multimodal Information** EΒ WB NB SB 85th % Speed / Rest in Walk / Corner Radius 25 0 25 0 25 0 25 0 No No No Nο 9.0 0 9.0 0 9.0 12 0 9.0 12 0 Walkway / Crosswalk Width / Length, ft 12 12 Street Width / Island / Curb 0 0 0 0 0 0 0 0 No No No No Width Outside / Bike Lane / Shoulder, ft 12 5.0 2.0 12 5.0 2.0 12 5.0 2.0 12 5.0 2.0

Pedestrian Signal / Occupied Parking

No

0.50

No

No

0.50

0.50

0.50

HCS 2010 Signalized Intersection Results Summary Intersection Information 14. 4177 **General Information** Agency Infrastructure Engineering, Inc. Duration, h 0.25 Analyst Jane Canada Analysis Date 1/11/2016 Area Type Other PHF 1.00 Jurisdiction Time Period Intersection SR 46 & Westwood Boulev Analysis Year 2016 **Analysis Period** 1> 7:00 AM Existing 2036 - Ricker's @ SR 46 & Westwood Boulevard - Columbus, IN.xus File Name **Project Description** WB **Demand Information** EB NB SB Approach Movement R L R R R 203 308 97 5 Demand (v), veh/h 833 6 37 71 2 11 91 44 **Signal Information** ؞؞ڶڶ؞ Cycle, s 84.3 Reference Phase 2 Offset, s 0 Reference Point Begin 60.0 0.0 0.0 0.0 Green 13.1 0.0 Uncoordinated Yes Simult. Gap E/W On Yellow 3.9 4.3 0.0 0.0 0.0 0.0 Force Mode Fixed Simult. Gap N/S 0.0 On Red 1.5 1.5 0.0 0.0 0.0 **Timer Results EBL EBT WBL** WBT NBL **NBT** SBL SBT **Assigned Phase** 4 8 2 6 Case Number 6.0 5.0 6.0 6.0 Phase Duration, s 65.8 65.8 18.5 18.5 Change Period, (Y+Rc), s 5.8 5.8 5.4 5.4 Max Allow Headway (MAH), s 4.2 4.2 2.8 2.8 7.0 Queue Clearance Time (gs), s 14.4 10.9 12.7 Green Extension Time (ge), s 6.8 6.8 0.3 0.3 Phase Call Probability 1.00 1.00 1.00 1.00 0.01 0.01 0.00 0.00 Max Out Probability WB NB SB **Movement Group Results** EΒ Approach Movement L Т R L Т R L Т R L Т R **Assigned Movement** 7 4 14 3 8 18 5 2 12 1 6 16 Adjusted Flow Rate (v), veh/h 203 420 419 37 308 71 2 108 91 49 1020 1881 1876 653 1792 1594 1378 1636 1306 1398 Adjusted Saturation Flow Rate (s), veh/h/ln 7.3 7.0 7.0 1.9 5.1 1.1 0.1 5.0 5.7 2.6 Queue Service Time (gs), s Cycle Queue Clearance Time (gc), s 12.4 7.0 7.0 8.9 5.1 1.1 2.6 5.0 10.7 2.6 749 1338 1335 1275 1134 259 255 212 495 218 Capacity (c), veh/h 0.429 Volume-to-Capacity Ratio (X) 0.271 0.314 0.314 0.075 0.242 0.063 0.008 0.423 0.225 Available Capacity (ca), veh/h 749 1338 1335 495 1275 1134 534 582 473 497 Back of Queue (Q), veh/ln (50th percentile) 1.4 2.0 2.0 0.2 0.3 0.0 2.0 1.8 8.0 1.3 Overflow Queue (Q3), veh/ln 0.0 0.0 0.0 0.0 0.0 0.0 0.0 0.0 0.0 0.0 Queue Storage Ratio (RQ) (50th percentile) 0.34 0.16 0.16 0.03 0.03 0.01 0.01 0.25 0.15 0.08 32.2 37.0 Uniform Delay (d1), s/veh 6.4 4.5 4.5 6.2 4.2 3.7 32.3 31.1 Incremental Delay (d2), s/veh 0.9 0.6 0.6 0.3 0.4 0.1 0.0 0.4 0.5 0.2 Initial Queue Delay (d3), s/veh 0.0 0.0 0.0 0.0 0.0 0.0 0.0 0.0 0.0 0.0 Control Delay (d), s/veh 7.3 5.1 5.1 6.5 4.7 3.8 32.3 32.6 37.5 31.3 Level of Service (LOS) Α Α Α Α Α Α С С D С 5.6 Α 4.7 Α 32.6 С 35.3 Approach Delay, s/veh / LOS D Intersection Delay, s/veh / LOS 9.5 Α **Multimodal Results** ΕB WB NB SB Pedestrian LOS Score / LOS Bicycle LOS Score / LOS

HCS 2010 Signalized Intersection Intermediate Values 1474174 **General Information Intersection Information** Agency Infrastructure Engineering, Inc. Duration, h 0.25 Analyst Jane Canada Analysis Date 1/11/2016 Area Type Other PHF 1.00 Jurisdiction Time Period Intersection SR 46 & Westwood Boulev Analysis Year 2016 **Analysis Period** 1> 7:00 AM Existing 2036 - Ricker's @ SR 46 & Westwood Boulevard - Columbus, IN.xus File Name **Project Description** WB **Demand Information** EB NB SB Approach Movement R L R L R R 203 308 97 5 Demand (v), veh/h 833 6 37 71 2 11 91 44 **Signal Information** ٨. Cycle, s 84.3 Reference Phase 2 Offset, s 0 Reference Point Begin Green 13.1 60.0 0.0 0.0 0.0 0.0 Uncoordinated Yes Simult. Gap E/W On Yellow 3.9 4.3 0.0 0.0 0.0 0.0 Force Mode Fixed Simult. Gap N/S Red 1.5 1.5 0.0 0.0 0.0 0.0 FB WB NB SB Saturation Flow / Delay R L Τ R L R L R L Т Τ Τ Lane Width Adjustment Factor (fw) 1.000 1.000 1.000 1.000 1.000 1.000 1.000 1.000 1.000 1.000 1.000 1.000 Heavy Vehicle Adjustment Factor (fHV) 1.000 0.990 1.000 0.980 0.943 0.990 1.000 1.000 1.000 1.000 0.855 1.000 Approach Grade Adjustment Factor (fg) 1.000 1.000 1.000 1.000 1.000 1.000 1.000 1.000 1.000 1.000 1.000 1.000 Parking Activity Adjustment Factor (f_p) 1.000 1.000 1.000 1.000 1.000 1.000 1.000 1.000 1.000 1.000 1.000 1.000 Bus Blockage Adjustment Factor (fbb) 1.000 1.000 1.000 1.000 1.000 1.000 1.000 1.000 1.000 1.000 1.000 1.000 Area Type Adjustment Factor (fa) 1.000 1.000 1.000 1.000 1.000 1.000 1.000 1.000 1.000 1.000 1.000 1.000 Lane Utilization Adjustment Factor (fLU) 1.000 1.000 1.000 1.000 1.000 1.000 1.000 1.000 1.000 1.000 1.000 1.000 0.000 0.000 0.000 0.000 Left-Turn Adjustment Factor (fLT) Right-Turn Adjustment Factor (fRT) 0.997 0.000 0.861 0.861 1.000 Left-Turn Pedestrian Adjustment Factor (fLpb) 1.000 1.000 1.000 Right-Turn Ped-Bike Adjustment Factor (*f*_{Rpb}) 1.000 1.000 1.000 1.000 Movement Saturation Flow Rate (s), veh/h 3731 1792 167 143 Platoon Ratio (R_p) 1.00 1.00 1.00 1.00 Proportion of Vehicles Arriving on Green (P) Incremental Delay Factor (k) 0.04 0.04 0.04 0.50 0.50 0.50 0.50 0.50 0.50 0.04 **Signal Timing / Movement Groups EBL** EBT/R **WBL** WBT/R **NBL** NBT/R SBL SBT/R Lost Time (t_L) 5.8 5.8 5.4 5.4 Green Ratio (g/C) 0.71 0.71 0.16 0.16 Permitted Saturation Flow Rate (sp), veh/h/ln 1020 1378 1306 653 Shared Saturation Flow Rate (ssh), veh/h/ln 60.0 60.0 13.2 13.2 Permitted Effective Green Time (g_p) , s Permitted Service Time (gu), s 54.9 53.0 10.6 8.2 Permitted Queue Service Time (qps), s 7.3 1.9 0.1 5.7 0.0 0.0 0.0 Time to First Blockage (gf), s 0.0 Queue Service Time Before Blockage (gfs), s Protected Right Saturation Flow (s_R), veh/h/ln 0 Protected Right Effective Green Time (g_R) , s 0.0 Multimodal FB **WB** NB SB Pedestrian Fw / Fv Pedestrian Fs / Fdelay Pedestrian Mcorner / Mcw Bicycle c b / d b

Bicycle Fw / Fv

No errors or warnings exist.

--- Comments ---

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HCS 2010 Signalized Intersection Input Data 14. **General Information Intersection Information** Agency Infrastructure Engineering, Inc. Duration, h 0.25 Analyst Jane Canada Analysis Date 1/11/2016 Area Type Other PHF 1.00 Jurisdiction Time Period Intersection SR 46 & Westwood Boulev Analysis Year 2016 **Analysis Period** 1> 7:00 AM Proposed 2036 - Ricker's @ SR 46 & Westwood Boulevard - Columbus, IN.xus File Name **Project Description** WB **Demand Information** EB NB SB Approach Movement R L R L R R 203 833 308 Demand (v), veh/h 8 51 71 3 12 113 91 6 44 **Signal Information** ٨, Cycle, s 85.3 Reference Phase 2 Offset, s 0 Reference Point Begin 60.0 0.0 0.0 0.0 Green 14.1 0.0 Uncoordinated Yes Simult. Gap E/W On Yellow 3.9 4.3 0.0 0.0 0.0 0.0 Force Mode Fixed Simult. Gap N/S On Red 1.5 1.5 0.0 0.0 0.0 0.0 **Traffic Information** EΒ WB NB SB Approach Movement R Τ R R L Τ L L Τ L R Demand (v), veh/h 203 833 8 308 71 12 113 91 44 Initial Queue (Qb), veh/h 0 0 0 0 0 0 0 0 0 0 0 0 Base Saturation Flow Rate (so), veh/h 1900 1900 1900 1900 1900 1900 1900 1900 1900 1900 1900 1900 Parking (Nm), man/h None None None None Heavy Vehicles (PHV), % 0 1 2 6 1 0 0 0 17 Ped / Bike / RTOR, /h 0 0 0 0 0 0 0 0 0 0 0 0 Buses (Nb), buses/h 0 0 0 0 0 0 0 0 0 0 0 0 3 3 3 3 3 3 3 3 3 3 3 3 Arrival Type (AT) 1.00 1.00 1.00 Upstream Filtering (I) 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 Lane Width (W), ft 12.0 12.0 12.0 12.0 12.0 12.0 12.0 12.0 12.0 100 310 200 1150 750 160 195 290 Turn Bay Length, ft 313 Grade (P_g) , % 0 0 0 0 0 0 0 0 0 0 0 0 Speed Limit, mi/h 45 45 45 45 45 45 35 35 35 40 40 40 **Phase Information** WBL WBT NBT SBL SBT **EBL EBT NBL** Maximum Green (G_{max}) or Phase Split, s 60.0 60.0 30.0 30.0 4.3 Yellow Change Interval (Y), s 4.0 4.0 4.3 4.0 3.9 4.0 3.9 Red Clearance Interval (Rc), s 1.0 1.5 1.0 1.5 1.0 1.5 1.0 1.5 Minimum Green (Gmin), s 5 10 5 10 5 7 5 7 Start-Up Lost Time (It), s 2.0 2.0 2.0 2.0 2.0 2.0 2.0 2.0 Extension of Effective Green (e), s 2.0 2.0 2.0 2.0 2.0 2.0 2.0 2.0 Passage (PT), s 2.0 3.0 2.0 3.0 2.0 1.5 2.0 1.5 Recall Mode Min Max Min Max Min Min Min Min **Dual Entry** No Yes No Yes No Yes Nο Yes 0.0 Walk (Walk), s 0.0 0.0 0.0 0.0 0.0 0.0 0.0 Pedestrian Clearance Time (PC), s 0.0 0.0 0.0 0.0 0.0 0.0 0.0 0.0 **Multimodal Information** EΒ WB NB SB 85th % Speed / Rest in Walk / Corner Radius 25 0 25 0 25 0 25 0 No No No Nο 9.0 0 9.0 0 9.0 12 0 9.0 12 0 Walkway / Crosswalk Width / Length, ft 12 12 Street Width / Island / Curb 0 0 0 0 0 0 0 0 No No No No Width Outside / Bike Lane / Shoulder, ft 12 5.0 2.0 12 5.0 2.0 12 5.0 2.0 12 5.0 2.0

Pedestrian Signal / Occupied Parking

No

0.50

No

No

0.50

0.50

0.50

No

Generated: 2/5/2016 9:34:49 AM

HCS 2010 Signalized Intersection Results Summary Intersection Information Jal. 4177 **General Information** Agency Infrastructure Engineering, Inc. Duration, h 0.25 Analyst Jane Canada Analysis Date 1/11/2016 Area Type Other PHF 1.00 Jurisdiction Time Period Intersection SR 46 & Westwood Boulev Analysis Year 2016 **Analysis Period** 1> 7:00 AM Proposed 2036 - Ricker's @ SR 46 & Westwood Boulevard - Columbus, IN.xus File Name **Project Description** WB **Demand Information** EB NB SB Approach Movement R L R L R R 203 308 6 Demand (v), veh/h 833 8 51 71 3 12 113 91 44 **Signal Information** ؞؞ڶڶ؞ Cycle, s 85.3 Reference Phase 2 Offset, s 0 Reference Point Begin 60.0 0.0 0.0 0.0 Green 14.1 0.0 Uncoordinated Yes Simult. Gap E/W On Yellow 3.9 4.3 0.0 0.0 0.0 0.0 Force Mode Fixed Simult. Gap N/S 0.0 On Red 1.5 1.5 0.0 0.0 0.0 **Timer Results EBL EBT WBL** WBT NBL **NBT** SBL SBT **Assigned Phase** 4 8 2 6 Case Number 6.0 5.0 6.0 6.0 Phase Duration, s 65.8 65.8 19.5 19.5 Change Period, (Y+Rc), s 5.8 5.8 5.4 5.4 Max Allow Headway (MAH), s 4.2 4.2 2.8 2.8 Queue Clearance Time (gs), s 14.9 12.1 7.9 13.7 Green Extension Time (ge), s 7.0 7.0 0.4 0.3 Phase Call Probability 1.00 1.00 1.00 1.00 0.01 0.01 0.00 0.00 Max Out Probability WB NB SB **Movement Group Results** EΒ Approach Movement L Т R L Т R L Т R L Т R **Assigned Movement** 7 4 14 3 8 18 5 2 12 1 6 16 50 Adjusted Flow Rate (v), veh/h 203 421 420 51 308 71 3 125 91 Adjusted Saturation Flow Rate (s), veh/h/ln 1020 1881 1875 651 1792 1594 1376 1634 1286 1402 7.6 7.3 7.3 2.8 5.3 1.2 0.2 5.9 5.9 2.6 Queue Service Time (gs), s Cycle Queue Clearance Time (gc), s 12.9 7.3 7.3 10.1 5.3 1.2 2.7 5.9 11.7 2.6 738 1322 1259 1120 272 210 233 1317 486 272 Capacity (c), veh/h 0.433 Volume-to-Capacity Ratio (X) 0.275 0.319 0.319 0.105 0.245 0.063 0.011 0.460 0.215 Available Capacity (ca), veh/h 738 1322 1317 486 1259 1120 526 574 448 492 Back of Queue (Q), veh/ln (50th percentile) 1.4 2.1 2.1 0.4 1.5 0.3 0.1 2.3 1.8 0.9 Overflow Queue (Q3), veh/ln 0.0 0.0 0.0 0.0 0.0 0.0 0.0 0.0 0.0 0.0 Queue Storage Ratio (RQ) (50th percentile) 0.36 0.17 0.17 0.05 0.03 0.01 0.01 0.29 0.16 0.08 37.4 Uniform Delay (d1), s/veh 6.9 4.9 4.9 6.8 4.6 4.0 31.9 32.1 30.8 Incremental Delay (d2), s/veh 0.9 0.6 0.6 0.4 0.5 0.1 0.0 0.5 0.5 0.2 Initial Queue Delay (d3), s/veh 0.0 0.0 0.0 0.0 0.0 0.0 0.0 0.0 0.0 0.0 Control Delay (d), s/veh 7.8 5.5 5.5 7.3 5.0 4.1 31.9 32.6 37.9 30.9 Level of Service (LOS) Α Α Α Α Α Α С С D С 6.0 Α 5.1 Α 32.6 С 35.4 Approach Delay, s/veh / LOS D Intersection Delay, s/veh / LOS 10.1 В **Multimodal Results** ΕB WB NB SB Pedestrian LOS Score / LOS Bicycle LOS Score / LOS

HCS 2010 Signalized Intersection Intermediate Values 1474174 **General Information Intersection Information** Agency Infrastructure Engineering, Inc. Duration, h 0.25 Analyst Jane Canada Analysis Date 1/11/2016 Area Type Other PHF 1.00 Jurisdiction Time Period Intersection SR 46 & Westwood Boulev Analysis Year 2016 **Analysis Period** 1> 7:00 AM Proposed 2036 - Ricker's @ SR 46 & Westwood Boulevard - Columbus, IN.xus File Name **Project Description** WB **Demand Information** EB NB SB Approach Movement R L R R R 203 308 6 Demand (v), veh/h 833 8 51 71 3 12 113 91 44 **Signal Information** ٨. Cycle, s 85.3 Reference Phase 2 517 Offset, s 0 Reference Point Begin Green 14.1 60.0 0.0 0.0 0.0 0.0 Uncoordinated Yes Simult. Gap E/W On Yellow 3.9 4.3 0.0 0.0 0.0 0.0 Force Mode Fixed Simult. Gap N/S Red 1.5 1.5 0.0 0.0 0.0 0.0 FB WB NB SB Saturation Flow / Delay R L Τ R L R L R L Т Τ Τ Lane Width Adjustment Factor (fw) 1.000 1.000 1.000 1.000 1.000 1.000 1.000 1.000 1.000 1.000 1.000 1.000 Heavy Vehicle Adjustment Factor (fHV) 1.000 0.990 1.000 0.980 0.943 0.990 1.000 1.000 1.000 1.000 0.855 1.000 Approach Grade Adjustment Factor (fg) 1.000 1.000 1.000 1.000 1.000 1.000 1.000 1.000 1.000 1.000 1.000 1.000 Parking Activity Adjustment Factor (f_p) 1.000 1.000 1.000 1.000 1.000 1.000 1.000 1.000 1.000 1.000 1.000 1.000 Bus Blockage Adjustment Factor (fbb) 1.000 1.000 1.000 1.000 1.000 1.000 1.000 1.000 1.000 1.000 1.000 1.000 Area Type Adjustment Factor (fa) 1.000 1.000 1.000 1.000 1.000 1.000 1.000 1.000 1.000 1.000 1.000 1.000 Lane Utilization Adjustment Factor (fLU) 1.000 1.000 1.000 1.000 1.000 1.000 1.000 1.000 1.000 1.000 1.000 1.000 0.000 0.000 0.000 0.000 Left-Turn Adjustment Factor (fLT) Right-Turn Adjustment Factor (fRT) 0.997 0.000 0.860 0.863 1.000 Left-Turn Pedestrian Adjustment Factor (fLpb) 1.000 1.000 1.000 Right-Turn Ped-Bike Adjustment Factor (*f*_{Rpb}) 1.000 1.000 1.000 1.000 Movement Saturation Flow Rate (s), veh/h 3720 1792 157 168 Platoon Ratio (R_p) 1.00 1.00 1.00 1.00 Proportion of Vehicles Arriving on Green (P) Incremental Delay Factor (k) 0.04 0.04 0.04 0.50 0.50 0.50 0.50 0.50 0.50 0.04 **Signal Timing / Movement Groups EBL** EBT/R **WBL** WBT/R **NBL** NBT/R SBL SBT/R Lost Time (t_L) 5.8 5.8 5.4 5.4 Green Ratio (g/C) 0.70 0.70 0.17 0.17 Permitted Saturation Flow Rate (sp), veh/h/ln 1020 1286 651 1376 Shared Saturation Flow Rate (ssh), veh/h/ln 60.0 60.0 14.2 14.2 Permitted Effective Green Time (g_p) , s Permitted Service Time (gu), s 54.7 52.6 11.6 8.4 Permitted Queue Service Time (qps), s 7.6 2.8 0.2 5.9 0.0 0.0 0.0 Time to First Blockage (gf), s 0.0 Queue Service Time Before Blockage (gfs), s Protected Right Saturation Flow (s_R), veh/h/ln 0 Protected Right Effective Green Time (g_R) , s 0.0 Multimodal FB **WB** NB SB Pedestrian Fw / Fv Pedestrian Fs / Fdelay Pedestrian Mcorner / Mcw

Bicycle c b / d b
Bicycle Fw / Fv

No errors or warnings exist.

--- Comments ---

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HCS 2010 Signalized Intersection Input Data 14. **General Information Intersection Information** Agency Infrastructure Engineering, Inc. Duration, h 0.25 Analyst Jane Canada Analysis Date 1/11/2016 Area Type Other PHF 1.00 Jurisdiction Time Period Intersection SR 46 & Westwood Boulev Analysis Year 2016 Analysis Period 1> 7:00 PM Existing 2036 - Ricker's @ SR 46 & Westwood Boulevard - Columbus, IN.xus File Name **Project Description** WB **Demand Information** EB NB SB Approach Movement R L R L R L R 94 494 24 679 64 52 Demand (v), veh/h 118 14 12 183 48 218 **Signal Information** ؞؞ڶڶ؞ Cycle, s 91.4 Reference Phase 2 Offset, s 0 Reference Point Begin 60.0 0.0 0.0 0.0 Green 20.2 0.0 Uncoordinated Yes Simult. Gap E/W On Yellow 3.9 4.3 0.0 0.0 0.0 0.0 Force Mode Fixed Simult. Gap N/S On Red 1.5 1.5 0.0 0.0 0.0 0.0 **Traffic Information** FB WB NB SB Approach Movement R Τ R R L Τ L L Τ L Т R Demand (v), veh/h 494 24 118 679 64 14 12 183 48 218 Initial Queue (Qb), veh/h 0 0 0 0 0 0 0 0 0 0 0 0 Base Saturation Flow Rate (so), veh/h 1900 1900 1900 1900 1900 1900 1900 1900 1900 1900 1900 1900 Parking (Nm), man/h None None None None Heavy Vehicles (PHV), % 0 1 2 6 1 0 0 0 17 Ped / Bike / RTOR, /h 0 0 0 0 0 0 0 0 0 0 0 0 Buses (Nb), buses/h 0 0 0 0 0 0 0 0 0 0 0 0 3 3 3 3 3 3 3 3 3 3 3 3 Arrival Type (AT) 1.00 1.00 1.00 Upstream Filtering (I) 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 Lane Width (W), ft 12.0 12.0 12.0 12.0 12.0 12.0 12.0 12.0 12.0 100 310 200 1150 750 160 195 290 Turn Bay Length, ft 313 Grade (P_g) , % 0 0 0 0 0 0 0 0 0 0 0 0 Speed Limit, mi/h 45 45 45 45 45 45 35 35 35 35 35 35 **Phase Information** WBL WBT NBT SBL SBT **EBL EBT** NBL Maximum Green (G_{max}) or Phase Split, s 60.0 60.0 30.0 30.0 4.3 Yellow Change Interval (Y), s 4.0 4.0 4.3 4.0 3.9 4.0 3.9 Red Clearance Interval (Rc), s 1.0 1.5 1.0 1.5 1.0 1.5 1.0 1.5 Minimum Green (Gmin), s 5 10 5 10 5 7 5 7 Start-Up Lost Time (It), s 2.0 2.0 2.0 2.0 2.0 2.0 2.0 2.0 Extension of Effective Green (e), s 2.0 2.0 2.0 2.0 2.0 2.0 2.0 2.0 Passage (PT), s 2.0 3.0 2.0 3.0 2.0 1.5 2.0 1.5 Recall Mode Min Max Min Max Min Min Min Min **Dual Entry** No Yes No Yes No Yes Nο Yes 0.0 Walk (Walk), s 0.0 0.0 0.0 0.0 0.0 0.0 0.0 Pedestrian Clearance Time (PC), s 0.0 0.0 0.0 0.0 0.0 0.0 0.0 0.0 **Multimodal Information** EΒ WB NB SB 85th % Speed / Rest in Walk / Corner Radius 25 25 0 25 0 25 0 No 0 No No Nο 9.0 0 9.0 0 9.0 12 0 9.0 12 0 Walkway / Crosswalk Width / Length, ft 12 12 Street Width / Island / Curb 0 0 0 0 0 0 0 0 No No No No Width Outside / Bike Lane / Shoulder, ft 12 5.0 2.0 12 5.0 2.0 12 5.0 2.0 12 5.0 2.0

Pedestrian Signal / Occupied Parking

No

0.50

No

No

0.50

0.50

0.50

No

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HCS 2010 Signalized Intersection Results Summary Intersection Information 14. 4177 **General Information** Agency Infrastructure Engineering, Inc. Duration, h 0.25 Analyst Jane Canada Analysis Date 1/11/2016 Area Type Other PHF 1.00 Jurisdiction Time Period Intersection SR 46 & Westwood Boulev Analysis Year 2016 **Analysis Period** 1> 7:00 PM Existing 2036 - Ricker's @ SR 46 & Westwood Boulevard - Columbus, IN.xus File Name **Project Description** WB **Demand Information** EB NB SB Approach Movement R L R L R L R 494 24 679 64 52 Demand (v), veh/h 94 118 14 12 183 48 218 **Signal Information** ؞؞ڶڶ؞ Cycle, s 91.4 Reference Phase 2 Offset, s 0 Reference Point Begin 60.0 0.0 0.0 0.0 Green 20.2 0.0 Uncoordinated Yes Simult. Gap E/W On Yellow 3.9 4.3 0.0 0.0 0.0 0.0 Force Mode Fixed Simult. Gap N/S 0.0 On Red 1.5 1.5 0.0 0.0 0.0 **Timer Results EBL EBT WBL** WBT NBL **NBT** SBL SBT **Assigned Phase** 4 8 2 6 Case Number 6.0 5.0 6.0 6.0 Phase Duration, s 65.8 65.8 25.6 25.6 Change Period, (Y+Rc), s 5.8 5.8 5.4 5.4 Max Allow Headway (MAH), s 4.2 4.2 2.8 2.8 Queue Clearance Time (gs), s 28.7 21.2 19.6 18.5 Green Extension Time (ge), s 7.2 7.4 0.6 0.6 1.00 Phase Call Probability 1.00 1.00 1.00 0.06 0.03 0.00 0.00 Max Out Probability WB NB SB **Movement Group Results** EΒ Approach Movement L Т R L Т R L Т R L Т R **Assigned Movement** 7 4 14 3 8 18 5 2 12 1 6 16 Adjusted Flow Rate (v), veh/h 94 261 257 118 679 64 14 64 183 266 728 1881 1850 880 1792 1594 1131 1658 1359 1415 Adjusted Saturation Flow Rate (s), veh/h/ln 7.5 5.1 5.7 19.2 1.1 2.9 16.5 Queue Service Time (gs), s 5.1 1.3 11.5 26.7 Cycle Queue Clearance Time (gc), s 5.1 5.1 10.7 19.2 1.3 17.6 2.9 14.4 16.5 404 1234 337 1214 607 1176 1046 125 367 314 Capacity (c), veh/h Volume-to-Capacity Ratio (X) 0.233 0.211 0.212 0.194 0.577 0.061 0.112 0.174 0.543 0.848 Available Capacity (ca), veh/h 404 1234 1214 607 1176 1046 246 544 482 464 Back of Queue (Q), veh/ln (50th percentile) 1.3 1.7 1.7 1.0 6.3 0.4 0.3 1.1 3.7 6.0 Overflow Queue (Q3), veh/ln 0.0 0.0 0.0 0.0 0.0 0.0 0.0 0.0 0.0 0.0 Queue Storage Ratio (RQ) (50th percentile) 0.32 0.14 0.14 0.13 0.14 0.01 0.05 0.14 0.32 0.55 28.8 Uniform Delay (d1), s/veh 16.1 6.3 6.3 8.4 8.7 5.6 42.6 34.7 34.1 Incremental Delay (d2), s/veh 1.3 0.4 0.4 0.7 2.1 0.1 0.1 0.1 0.5 6.3 Initial Queue Delay (d3), s/veh 0.0 0.0 0.0 0.0 0.0 0.0 0.0 0.0 0.0 0.0 Control Delay (d), s/veh 17.4 6.7 6.7 9.1 10.8 5.7 42.7 28.9 35.2 40.4 Level of Service (LOS) В Α Α Α В Α D С D D 8.3 Α 10.2 В 31.4 С 38.3 Approach Delay, s/veh / LOS D Intersection Delay, s/veh / LOS 16.8 В **Multimodal Results** ΕB WB NB SB Pedestrian LOS Score / LOS Bicycle LOS Score / LOS

HCS 2010 Signalized Intersection Intermediate Values 1474174 **General Information Intersection Information** Agency Infrastructure Engineering, Inc. Duration, h 0.25 Analyst Jane Canada Analysis Date 1/11/2016 Area Type Other PHF 1.00 Jurisdiction Time Period Intersection SR 46 & Westwood Boulev Analysis Year 2016 **Analysis Period** 1> 7:00 PM Existing 2036 - Ricker's @ SR 46 & Westwood Boulevard - Columbus, IN.xus File Name **Project Description** WB **Demand Information** EB NB SB Approach Movement R L R L R L R 494 24 118 679 64 52 Demand (v), veh/h 94 14 12 183 48 218 **Signal Information** د لله Cycle, s 91.4 Reference Phase 2 517 Offset, s 0 Reference Point Begin Green 20.2 60.0 0.0 0.0 0.0 0.0 Uncoordinated Yes Simult. Gap E/W On Yellow 3.9 4.3 0.0 0.0 0.0 0.0 Force Mode Fixed Simult. Gap N/S Red 1.5 1.5 0.0 0.0 0.0 0.0 FB WB NB SB Saturation Flow / Delay R L Τ R L R L R L Т Τ Τ Lane Width Adjustment Factor (fw) 1.000 1.000 1.000 1.000 1.000 1.000 1.000 1.000 1.000 1.000 1.000 1.000 Heavy Vehicle Adjustment Factor (fHV) 1.000 0.990 1.000 0.980 0.943 0.990 1.000 1.000 1.000 1.000 0.855 1.000 Approach Grade Adjustment Factor (fg) 1.000 1.000 1.000 1.000 1.000 1.000 1.000 1.000 1.000 1.000 1.000 1.000 Parking Activity Adjustment Factor (f_p) 1.000 1.000 1.000 1.000 1.000 1.000 1.000 1.000 1.000 1.000 1.000 1.000 Bus Blockage Adjustment Factor (fbb) 1.000 1.000 1.000 1.000 1.000 1.000 1.000 1.000 1.000 1.000 1.000 1.000 Area Type Adjustment Factor (fa) 1.000 1.000 1.000 1.000 1.000 1.000 1.000 1.000 1.000 1.000 1.000 1.000 Lane Utilization Adjustment Factor (fLU) 1.000 1.000 1.000 1.000 1.000 1.000 1.000 1.000 1.000 1.000 1.000 1.000 0.000 0.000 0.000 0.000 Left-Turn Adjustment Factor (fLT) Right-Turn Adjustment Factor (fRT) 0.983 0.000 0.872 0.871 1.000 Left-Turn Pedestrian Adjustment Factor (fLpb) 1.000 1.000 1.000 Right-Turn Ped-Bike Adjustment Factor (*f*_{Rpb}) 1.000 1.000 1.000 1.000 Movement Saturation Flow Rate (s), veh/h 3560 1792 311 255 Platoon Ratio (R_p) 1.00 1.00 1.00 1.00 Proportion of Vehicles Arriving on Green (P) Incremental Delay Factor (k) 0.04 0.04 0.50 0.50 0.50 0.50 0.50 0.50 0.04 0.11 **Signal Timing / Movement Groups EBL** EBT/R **WBL** WBT/R **NBL** NBT/R SBL SBT/R Lost Time (t_L) 5.8 5.8 5.4 5.4 Green Ratio (g/C) 0.66 0.66 0.22 0.22 Permitted Saturation Flow Rate (sp), veh/h/ln 728 1359 880 1131 Shared Saturation Flow Rate (ssh), veh/h/ln 60.0 60.0 20.3 20.3 Permitted Effective Green Time (g_p) , s Permitted Service Time (gu), s 40.8 54.9 3.8 17.4 Permitted Queue Service Time (qps), s 7.5 5.7 1.1 11.5 0.0 0.0 Time to First Blockage (gf), s 0.0 0.0 Queue Service Time Before Blockage (gfs), s Protected Right Saturation Flow (s_R), veh/h/ln 0 Protected Right Effective Green Time (g_R) , s 0.0 Multimodal FB **WB** NB SB Pedestrian Fw / Fv Pedestrian Fs / Fdelay Pedestrian Mcorner / Mcw Bicycle c b / d b

Bicycle Fw / Fv

No errors or warnings exist.

--- Comments ---

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HCS 2010 Signalized Intersection Input Data 14. **General Information Intersection Information** Agency Infrastructure Engineering, Inc. Duration, h 0.25 Analyst Jane Canada Analysis Date 1/11/2016 Area Type Other PHF 1.00 Jurisdiction Time Period Intersection SR 46 & Westwood Boulev Analysis Year 2016 **Analysis Period** 1> 7:00 PM Proposed 2036 - Ricker's @ SR 46 & Westwood Boulevard - Columbus, IN.xus File Name **Project Description** WB **Demand Information** EB NB SB Approach Movement R L R L R L R 94 494 28 64 Demand (v), veh/h 139 679 21 13 77 183 56 218 **Signal Information** ؞؞ڶڶ؞ Cycle, s 90.2 Reference Phase 2 Offset, s 0 Reference Point Begin 60.0 0.0 0.0 0.0 Green 19.0 0.0 Uncoordinated Yes Simult. Gap E/W On Yellow 3.9 4.3 0.0 0.0 0.0 0.0 Force Mode Fixed Simult. Gap N/S On Red 1.5 1.5 0.0 0.0 0.0 0.0 **Traffic Information** FB WB NB SB Approach Movement R Τ R R L Τ L L Τ L Τ R Demand (v), veh/h 494 28 139 679 64 21 13 77 183 56 218 Initial Queue (Qb), veh/h 0 0 0 0 0 0 0 0 0 0 0 0 Base Saturation Flow Rate (so), veh/h 1900 1900 1900 1900 1900 1900 1900 1900 1900 1900 1900 1900 Parking (Nm), man/h None None None None Heavy Vehicles (PHV), % 0 0 0 0 0 0 0 0 0 Ped / Bike / RTOR, /h 0 0 0 0 0 0 0 0 0 0 0 0 Buses (Nb), buses/h 0 0 0 0 0 0 0 0 0 0 0 0 3 3 3 3 3 3 3 3 3 3 3 3 Arrival Type (AT) 1.00 1.00 1.00 Upstream Filtering (I) 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 Lane Width (W), ft 12.0 12.0 12.0 12.0 12.0 12.0 12.0 12.0 12.0 100 310 200 1150 750 160 195 290 Turn Bay Length, ft 313 Grade (P_g) , % 0 0 0 0 0 0 0 0 0 0 0 0 Speed Limit, mi/h 45 45 45 45 45 45 35 35 35 40 40 40 **Phase Information** WBL WBT NBT SBL SBT **EBL EBT NBL** Maximum Green (G_{max}) or Phase Split, s 60.0 60.0 35.0 35.0 4.3 Yellow Change Interval (Y), s 4.0 4.0 4.3 4.0 3.9 4.0 3.9 Red Clearance Interval (Rc), s 1.0 1.5 1.0 1.5 1.0 1.5 1.0 1.5 Minimum Green (Gmin), s 5 10 5 10 5 7 5 7 Start-Up Lost Time (It), s 2.0 2.0 2.0 2.0 2.0 2.0 2.0 2.0 Extension of Effective Green (e), s 2.0 2.0 2.0 2.0 2.0 2.0 2.0 2.0 Passage (PT), s 2.0 3.0 2.0 3.0 2.0 1.5 2.0 1.5 Recall Mode Min Max Min Max Min Min Min Min **Dual Entry** No Yes No Yes No Yes Nο Yes 0.0 Walk (Walk), s 0.0 0.0 0.0 0.0 0.0 0.0 0.0 Pedestrian Clearance Time (PC), s 0.0 0.0 0.0 0.0 0.0 0.0 0.0 0.0 **Multimodal Information** EΒ WB NB SB 85th % Speed / Rest in Walk / Corner Radius 25 0 25 0 25 0 25 0 No No No Nο 9.0 0 9.0 0 9.0 12 0 9.0 12 0 Walkway / Crosswalk Width / Length, ft 12 12 Street Width / Island / Curb 0 0 0 0 0 0 0 0 No No No No Width Outside / Bike Lane / Shoulder, ft 12 5.0 2.0 12 5.0 2.0 12 5.0 2.0 12 5.0 2.0

Pedestrian Signal / Occupied Parking

No

0.50

No

No

0.50

0.50

0.50

No

Generated: 2/5/2016 9:36:13 AM

HCS 2010 Signalized Intersection Results Summary Intersection Information Jal. 4177 **General Information** Agency Infrastructure Engineering, Inc. Duration, h 0.25 Analyst Jane Canada Analysis Date 1/11/2016 Area Type Other PHF 1.00 Jurisdiction Time Period Intersection SR 46 & Westwood Boulev Analysis Year 2016 **Analysis Period** 1> 7:00 PM Proposed 2036 - Ricker's @ SR 46 & Westwood Boulevard - Columbus, IN.xus File Name **Project Description** WB **Demand Information** EB NB SB Approach Movement R L R L R L R 494 28 64 Demand (v), veh/h 94 139 679 21 13 77 183 56 218 **Signal Information** ؞؞ڶڶ؞ Cycle, s 90.2 Reference Phase 2 Offset, s 0 Reference Point Begin 60.0 0.0 0.0 0.0 Green 19.0 0.0 Uncoordinated Yes Simult. Gap E/W On Yellow 3.9 4.3 0.0 0.0 0.0 0.0 Force Mode Fixed Simult. Gap N/S 0.0 On Red 1.5 1.5 0.0 0.0 0.0 **Timer Results EBL EBT WBL** WBT NBL **NBT** SBL SBT **Assigned Phase** 4 8 2 6 Case Number 6.0 5.0 6.0 6.0 Phase Duration, s 65.8 65.8 24.4 24.4 Change Period, (Y+Rc), s 5.8 5.8 5.4 5.4 Max Allow Headway (MAH), s 4.2 4.2 2.8 2.8 Queue Clearance Time (gs), s 25.8 18.8 17.7 18.2 Green Extension Time (ge), s 7.5 7.6 0.7 0.7 1.00 Phase Call Probability 1.00 1.00 1.00 0.05 0.02 0.00 0.00 Max Out Probability WB NB SB **Movement Group Results** ΕB Approach Movement L Т R L Т R L Т R L Т R **Assigned Movement** 7 4 14 3 8 18 5 2 12 1 6 16 Adjusted Flow Rate (v), veh/h 94 263 259 139 679 64 21 90 183 274 728 1900 1864 894 1900 1610 1123 1646 1327 1662 Adjusted Saturation Flow Rate (s), veh/h/ln 7.0 4.9 6.5 16.8 1.6 4.1 12.0 14.1 Queue Service Time (gs), s 4.9 1.3 Cycle Queue Clearance Time (gc), s 23.8 4.9 4.9 11.3 16.8 1.3 15.7 4.1 16.2 14.1 428 1263 1239 626 1071 142 347 299 1263 351 Capacity (c), veh/h 0.222 Volume-to-Capacity Ratio (X) 0.220 0.208 0.209 0.537 0.060 0.148 0.259 0.611 0.781 Available Capacity (ca), veh/h 428 1263 1239 626 1263 1071 340 639 534 645 Back of Queue (Q), veh/ln (50th percentile) 1.2 1.6 1.6 1.2 5.7 0.4 0.4 1.6 3.8 5.5 Overflow Queue (Q3), veh/ln 0.0 0.0 0.0 0.0 0.0 0.0 0.0 0.0 0.0 0.0 Queue Storage Ratio (RQ) (50th percentile) 0.29 0.13 0.13 0.15 0.12 0.01 0.07 0.21 0.33 0.44 29.7 36.5 Uniform Delay (d1), s/veh 14.1 5.9 5.9 8.1 7.9 5.3 41.0 33.6 Incremental Delay (d2), s/veh 1.2 0.4 0.4 8.0 1.6 0.1 0.2 0.1 8.0 1.4 Initial Queue Delay (d3), s/veh 0.0 0.0 0.0 0.0 0.0 0.0 0.0 0.0 0.0 0.0 Control Delay (d), s/veh 15.3 6.3 6.3 8.9 9.5 5.4 41.2 29.9 37.2 35.1 Level of Service (LOS) В Α Α Α Α Α D С D D 7.6 Α 9.1 Α 32.0 С 35.9 Approach Delay, s/veh / LOS D Intersection Delay, s/veh / LOS В 15.8 **Multimodal Results** ΕB WB NB SB Pedestrian LOS Score / LOS Bicycle LOS Score / LOS

HCS 2010 Signalized Intersection Intermediate Values 1474174 **General Information Intersection Information** Agency Infrastructure Engineering, Inc. Duration, h 0.25 Analyst Jane Canada Analysis Date 1/11/2016 Area Type Other PHF 1.00 Jurisdiction Time Period Intersection SR 46 & Westwood Boulev Analysis Year 2016 **Analysis Period** 1> 7:00 PM Proposed 2036 - Ricker's @ SR 46 & Westwood Boulevard - Columbus, IN.xus File Name **Project Description** WB **Demand Information** EB NB SB Approach Movement R L R L R L R 28 64 Demand (v), veh/h 94 494 139 679 21 13 77 183 56 218 **Signal Information** ىل:، Cycle, s 90.2 Reference Phase 2 517 Offset, s 0 Reference Point Begin Green 19.0 60.0 0.0 0.0 0.0 0.0 Uncoordinated Yes Simult. Gap E/W On Yellow 3.9 4.3 0.0 0.0 0.0 0.0 Force Mode Fixed Simult. Gap N/S On Red 1.5 1.5 0.0 0.0 0.0 0.0 FB WB NB SB Saturation Flow / Delay R L Τ R L R L R L Т Τ Τ Lane Width Adjustment Factor (fw) 1.000 1.000 1.000 1.000 1.000 1.000 1.000 1.000 1.000 1.000 1.000 1.000 Heavy Vehicle Adjustment Factor (fHV) 1.000 1.000 1.000 1.000 1.000 1.000 1.000 1.000 1.000 1.000 1.000 1.000 Approach Grade Adjustment Factor (fg) 1.000 1.000 1.000 1.000 1.000 1.000 1.000 1.000 1.000 1.000 1.000 1.000 Parking Activity Adjustment Factor (f_p) 1.000 1.000 | 1.000 1.000 1.000 1.000 1.000 1.000 1.000 1.000 1.000 1.000 Bus Blockage Adjustment Factor (fbb) 1.000 1.000 1.000 1.000 1.000 1.000 1.000 1.000 1.000 1.000 1.000 1.000 Area Type Adjustment Factor (fa) 1.000 1.000 1.000 1.000 1.000 1.000 1.000 1.000 1.000 1.000 1.000 1.000 Lane Utilization Adjustment Factor (fLU) 1.000 1.000 1.000 1.000 1.000 1.000 1.000 1.000 1.000 1.000 1.000 1.000 0.000 0.000 0.000 0.000 Left-Turn Adjustment Factor (fLT) 0.981 0.000 0.867 0.875 Right-Turn Adjustment Factor (frt) 1.000 Left-Turn Pedestrian Adjustment Factor (fLpb) 1.000 1.000 1.000 Right-Turn Ped-Bike Adjustment Factor (*f*_{Rpb}) 1.000 1.000 1.000 1.000 Movement Saturation Flow Rate (s), veh/h 3564 1900 238 340 Platoon Ratio (R_p) 1.00 1.00 1.00 1.00 Proportion of Vehicles Arriving on Green (P) Incremental Delay Factor (k) 0.04 0.04 0.04 0.50 0.50 0.50 0.50 0.50 0.50 0.04 **Signal Timing / Movement Groups EBL** EBT/R **WBL** WBT/R **NBL** NBT/R SBL SBT/R Lost Time (t_L) 5.8 5.8 5.4 5.4 Green Ratio (g/C) 0.67 0.67 0.21 0.21 Permitted Saturation Flow Rate (sp), veh/h/ln 728 1123 1327 894 Shared Saturation Flow Rate (ssh), veh/h/ln 60.0 60.0 19.0 19.0 Permitted Effective Green Time (g_p) , s Permitted Service Time (gu), s 43.2 55.1 5.0 14.9 Permitted Queue Service Time (qps), s 7.0 12.0 6.5 1.6 0.0 0.0 0.0 Time to First Blockage (gf), s 0.0 Queue Service Time Before Blockage (gfs), s Protected Right Saturation Flow (s_R), veh/h/ln 0 Protected Right Effective Green Time (g_R) , s 0.0 Multimodal FB **WB** NB SB Pedestrian Fw / Fv Pedestrian Fs / Fdelay Pedestrian Mcorner / Mcw Bicycle c b / d b

Bicycle Fw / Fv

No errors or warnings exist.

--- Comments ---

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HCS 2010™ Streets Version 6.41

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Appendix F - INDOT Signal Timing

Configuration

				Cont	rolle	er Seg	uence	Prio	rity			
	1	2	3	4	5	6	7	8	9	10	11	12
Ring 1 Phases Ring 2 Phases	1 5	2 6	3 7	4 8	9 11	10 12	0 0	0 0	0	0 0	0 0	0
							Phase					
	1	2	3	4	5	6	7	8	9	10	11	12
In Use												
Exclusive Ped Direction	•	•	•	•	•	•	•	•	٠	•	•	•

Overlap A B C D

Direction . . .

Load Switch Channel/Driver Group Assign (Info Only):

ьоас	ı				Signal	Signal				
Swite	ch				Driver	Group				
JMM)										
Chanr	ne]	L			Ovlap	Ped				
1					1					
2					2					
3					3	•				
4					4	•				
5					5	•				
6					6	•				
7					7	•				
8					8	•				
9					9	•				
10					10	•				
11					11					
12					12	•				
13					А	•				
14					В					
15					С					
16					D					

```
Bartholomew County SR 46 @ 325 W 2/3/2016 10:04
Configuration Continued
______
             Enable BIU: 1 2 3 4 5 6 7 8
Terminal/Facilities. . . X . . . . . . . .
Detector Rack. . . . . X . . . . . . . .
Type 2 Runs as Type 1. . . .
MMU Disable. . . . . . . .
Diagnostic Enable. . . . .
Peer-Peer Comm Enable. . . .
                              2
                                  3
                                      4
                                          5
                                               6
                                                                255
Peer To Peer Addresses . . 255 255 255 255 255 255
                                                       255
                                                           255
Port 2:
Port 2 Protocol . . . . . . Terminal
Port 2 Enable . . . . . . . YES
AB3418 Address. . . . . . . . 0
AB3418 Group Address. . . . . 0
AB3418 Response Delay . . . . 0
AB3418 Single Flag Enable . . . NO
AB3418 Drop-Out Time. . . . . 0
AB3418 TOD SF Select. . . . . 0
Data Rate . . . . . . . . . . . . . 1200 bps
Data, Parity, Stop. . . . . . 8, 0, 1
Port 3:
Port 3 Protocol . . . . . . Telemetry
Port 3 Enable . . . . . . . . NO
Telemetry Address . . . . . . 0
System Detector 9-16 Address. . 0
Telemetry Response Delay. . . . 2000
```

AB3418 Address. 0
AB3418 Group Address. 0
AB3418 Response Delay 0
AB3418 Single Flag Enable . . NO
AB3418 Drop-Out Time. 0
AB3418 TOD SF Select. Full
Data Rate 9600 bps
Data, Parity, Stop. 8, 0, 1

Configuration Continued

Configuration Continued											
Event Enabling	Alarm Enabling										
Critical RFE'S (MMU/TF) X Non-Critical RFE'S (DET/TEST) . X Detector Errors X Coordination Errors X MMU Flash Faults X Local Flash Faults X Preempt X Power On/Off X Low Battery X	ALARM 1										
Supervisor Access Code **** Data Change Access Code ****											
MMU Compatibility Program (Info Only)											
Channel Is Allowed to Tire 16 15 14 13 12 11 10 9 1	8 7 6 5 4 3 2 · · · · · · · · · · · · · · · · · · ·										
4	x										

Channel				Is A	A110	owed	l to	Тi	me	Wit	h C	han	nel		
	16	15	14	13	12	11	10	9	8	7	6	5	4	3	2
1															
2											X				
3	•				•	•	•			•		•			
4	•				•	•	•		X	•		•			
5	•				•	•	•			•					
6	•				•	•	•			•					
7	•				•	•	•								
8						•									
9															
10															
11															
12															
13															
14															
15															

Version Info:		
Software Assy.	Part No.	Version
Boot	27831	2.83
Program	45561	7.9
Application		. 3
Help	27891	6.33
Configuration	27918	C000

By-Phase Timing Data

Direction	1	2	3	4	5	Ph 6	ase 7	8	9	10	11	12
Minimum Green	0	10	0	7	0	10	0	7	0	0	0	0
Bike Min Green	0	0	0	0	0	0	0	0	0	0	0	0
Cond Serv Min Grn	0	0	0	0	0	0	0	0	0	0	0	0
Walk	0	0	0	0	0	0	0	0	0	0	0	0
Ped Clearance	0	7	0	7	0	7	0	7	0	0	0	0
Veh Extension	0.0	5.0	0.0	3.0	0.0	5.0	0.0	3.0	0.0	0.0	0.0	0.0
Alt Veh Exten	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Max Extension	0	0	0	0	0	0	0	0	0	0	0	0
Max 1	0	60	0	30	0	60	0	30	0	0	0	0
Max 2	0	60	0	30	0	60	0	30	0	0	0	0
Max 3	0	0	0	0	0	0	0	0	0	0	0	0
Det. Fail Max	0	0	0	0	0	0	0	0	0	0	0	0
Yellow Change	3.0	4.3	3.0	3.9	3.0	4.3	3.0	3.9	3.0	3.0	3.0	3.0
Red Clearance	0.0	1.5	0.0	1.5	0.0	1.5	0.0	1.5	0.0	0.0	0.0	0.0
Red Revert	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Act. B4 Init	0	0	0	0	0	0	0	0	0	0	0	0
Sec/Actuation	0.0	2.0	0.0	0.0	0.0	2.0	0.0	0.0	0.0	0.0	0.0	0.0
Max Initial	0	30	0	0	0	30	0	0	0	0	0	0
Time B4 Reduction	0	15	0	9	0	15	0	9	0	0	0	0
Cars Waiting	0	0	0	0	0	0	0	0	0	0	0	0
Time To Reduce	0	15	0	9	0	15	0	9	0	0	0	0
Minimum Gap	0.0	3.0	0.0	1.5	0.0	3.0	0.0	1.5	0.0	0.0	0.0	0.0

No-Serve Phases

				Phase	Ca	nnot	Ser	ve Wi	th	Phase		
Phase	5	12	11	10	9	8	7	6	5	4	3	2
1.				•					•			
2.												
3.				•								
4.												
5.												
6.												
7.				•								
8.				•								
9.				•								
10.												
11.												

Ped Carryover

Ped Start Phase	Carry Over Phase	
1	0	
2	0	
3	0	
4	0	
5	0	
6	0	
7	0	
8	0	
9	0	
10	0	
11	0	
12	0	

Vehicle/Ped Phase as Overlap

				Ped	Pha	se A	s Ove	erlag	<u>S</u>			
Ped				Cons	ists	of :	Ped I	hase	es			
Ovlap												
Phase	1	2	3	4	5	6	7	8	9	10	11	12
1	•											
2		•	•	•				•			•	
3		•	•	•				•			•	
4			•	•	•	•		•				
5			•	•				•			•	
6			•	•				•			•	
7			•	•				•			•	
8			•	•				•			•	
9		•						•				
10		•						•				
11		•										
12		•										

Veh Phase As Overlap Consists of Veh Phases

Veh				Cons	ists	of	Veh	Phase	es			
Ovlap												
Phase	1	2	3	4	5	6	7	8	9	10	11	12
1	X			•								
2	•	X						•			•	
3	•		X					•			•	
4		•		X								
5	•			•	X			•			•	
6			•	•	•	X	•	•	•	•	•	•
7			•	•	•	•	X	•	•	•	•	•
8			•	•	•	•	•	X	•	•	•	•
9	•			•	•			•	X		•	
10	•			•	•			•		X	•	
11	•			•	•			•			X	
12	•										•	X

Overlap Data

overrap baca													
Overlap A				3	4	5	6	7	8	9	10	11	12
Standard													
Protected													
Permitted								•	•		•	•	
Enable Lag								•	•		•	•	
Enable Lead								•	•		•	•	
Spare								•	•		•	•	
Advance Green Time	er				0.0								
					Green					Red			
Lag/Lead Timers .					0.0		0.	0		0.0			
Overlap B	Phase:	1	2	3	4	5	6	7	8	9	10	11	12
Standard						_	-			_	_		
Protected													
Permitted													
Enable Lag													
Enable Lead													
Spare													
Advance Green Time													
					Green		Yel	low		Red			
Lag/Lead Timers .					0.0		0.	0		0.0			
Overlap C	Phase:	1	2	3	4	5	6	7	8	9	10	11	12
Standard						_	-		_	_	_		
Protected													
Permitted													
Enable Lag													
Enable Lead													
Spare													
Advance Green Time													
					Green		Yel	low		Red			
Lag/Lead Timers .					0.0		0.	0		0.0			
-													
Overlap D	Phase:	1	2	3	4	5	6	7	8	9	10	11	12
Standard		_		,	<u>.</u>	_		,	5		± 0		
Protected		•	•	•	•	•	•	•	•	•	•	•	•
Permitted		•	•	•	•	•	•	•	•	•	•	•	•
Enable Lag		•	•	•	•	•	•	•	•	•	•	•	•
Enable Lead		•	•	•	•	•	•	•	•	•	•	•	•
Spare		•	•	•	•	•	•	•	•	•	•	•	•
Advance Green Time			•		0.0	•	•	•	•	•	•	•	•
	• •	• •	• •	•	Green		уеl	low		Red			
					J_ CC11	•							

Lag/Lead Timers 0.0 0.0 0.0

Power Start, Remote Flash _____ Phase 1 2 3 4 5 6 7 8 9 10 11 12 Power Start. X . . X Χ. External Start X . . . Into Remote Flash. . X . . X . . X . Exit Remote Flash. . Χ. Overlap Remote Flash Yellow. A B C D Initialization Interval: Power Start Green External Start. Green Power Start All Red Time. . Power Start Flash Time. . . 5 Remote Flash Options: Out of Flash Yellow NO Out of Flash All Red. . . . NO

NO

Bartholomew County SR 46 @ 325 W 2/3/2016 10:04

Minimum Recall. NO Alternate Flash NO Flash Thru Load Switches. . NO Cycle Through Phases. . . .

Option Data _____ Phase 1 2 3 4 5 6 7 8 9 10 11 12 Call To NonActuated 2 Dual Entry. X . . . X . Actuated Rest in Walk Enable Programmable Options Dual Entry. ON Backup Protection Group 1 OFF Conditional Service OFF Backup Protection Group 2 . . . OFF Ped Clearance Protection . . . OFF Backup Protection Group 3 . . . OFF Special Preempt Overlap Flash . OFF Simultaneous Gap Group 1 ON Cond Service Det Cross Switch . OFF Simultaneous Gap Group 2 ON Lock Detectors in Red Only. . . OFF Simultaneous Gap Group 3. . . . OFF

Five Section Left Turn Control Phases: 5-2 7-4 1-6 3-8 11-10 9-12 Left Turn Head.

Bartholomew County SR 46 @ 325 W 2/3/2016 10:04

Recall Data, Dimming

					P	has	e					
	1	2	3	4	5	6	7	8	9	10	11	12
Locking Detector		X				X						
Vehicle Recall		X				X						
Pedestrian Recall												
Recall To Max												
Soft Recall												
Don't Rest Here												
Ped Dark if No Call												

Dimming:

Load Switch

	1	2	3	4	5	6	7	8	9	10	11	12	13	14	15	16
Green/Walk	NO															
Yellow/Ped Clear.	NO															
Red/Don't Walk	NO															

Detector Type/Timers

	Locking	Log	Tim	ers	Don't Res	et	
Det.	Memory	Enable	Extend	Delay	Extend		Type
1	NO	NO	0.0	7		1	- Extend/Delay
2	NO	YES	0.0	7	•	1	- Extend/Delay
3	NO	NO	0.0	3	•	1	- Extend/Delay
4	NO	YES	0.0	3	•	1	- Extend/Delay
5	NO	YES	0.0	0	•	1	- Extend/Delay
6	NO	YES	0.0	0	•	1	Extend/Delay
7	NO	NO	0.0	0	•	1	Extend/Delay
8	NO	YES	0.0	0	•	1	Extend/Delay
9	NO	NO	0.0	7	•	1	- Extend/Delay
10	NO	YES	0.0	7	•	1	- Extend/Delay
11	NO	NO	0.0	3	•	1	- Extend/Delay
12	NO	YES	0.0	3	•	1	 Extend/Delay
13	NO	NO	0.0	0	•	1	 Extend/Delay
14	NO	YES	0.0	0	•	1	 Extend/Delay
15	NO	YES	0.0	0	•	1	 Extend/Delay
16	NO	YES	0.0	0	•	1	 Extend/Delay
17	NO	NO	0.0	0	•	0	- Normal
18	NO	NO	0.0	0	•	0	- Normal
19	NO	NO	0.0	0	•	0	- Normal
20	NO	NO	0.0	0	•	0	- Normal
21	NO	NO	0.0	0	•	0	- Normal
22	NO	NO	0.0	0	•	0	- Normal
23	NO	NO	0.0	0	•	0	- Normal
24	NO	NO	0.0	0	•	0	- Normal
25	NO	NO	0.0	0	•	0	- Normal
26	NO	NO	0.0	0	•	0	- Normal
27	NO	NO	0.0	0	•	0	- Normal
28	NO	NO	0.0	0	•	0	- Normal
29	NO	NO	0.0	0	•	0	- Normal
30	NO	NO	0.0	0	•	0	- Normal
31	NO	NO	0.0	0	•	0	- Normal
32	NO	NO	0.0	0	•	0	- Normal

Detector Names

Det	1:	Detector	1	Det 17: Detector	17
Det	2:	Detector	2	Det 18: Detector	18
Det	3:	Detector	3	Det 19: Detector	19
Det	4:	Detector	4	Det 20: Detector	20
Det	5:	Detector	5	Det 21: Detector	21
Det	6:	Detector	6	Det 22: Detector	22
Det	7:	Detector	7	Det 23: Detector	23
Det	8:	Detector	8	Det 24: Detector	24
Det	9:	Detector	9	Det 25: Detector	25
Det	10:	Detector	10	Det 26: Detector	26
Det	11:	Detector	11	Det 27: Detector	27
Det	12:	Detector	12	Det 28: Detector	28
Det	13:	Detector	13	Det 29: Detector	29
Det	14:	Detector	14	Det 30: Detector	30
Det	15:	Detector	15	Det 31: Detector	31
Det	16:	Detector	16	Det 32: Detector	32

Detector Type/Timers

33	NO	NO	0.0	0	•	0 - Normal
34	NO	NO	0.0	0	•	0 - Normal
35	NO	NO	0.0	0		0 - Normal
36	NO	NO	0.0	0		0 - Normal
37	NO	NO	0.0	0	•	0 - Normal
38	NO	NO	0.0	0	•	0 - Normal
39	NO	NO	0.0	0	•	0 - Normal
40	NO	NO	0.0	0		0 - Normal
41	NO	NO	0.0	0		0 - Normal
42	NO	NO	0.0	0		0 - Normal
43	NO	NO	0.0	0		0 - Normal
44	NO	NO	0.0	0		0 - Normal
45	NO	NO	0.0	0		0 - Normal
46	NO	NO	0.0	0		0 - Normal
47	NO	NO	0.0	0		0 - Normal
48	NO	NO	0.0	0		0 - Normal
49	NO	NO	0.0	0		0 - Normal
50	NO	NO	0.0	0		0 - Normal
51	NO	NO	0.0	0		0 - Normal
52	NO	NO	0.0	0		0 - Normal
53	NO	NO	0.0	0		0 - Normal
54	NO	NO	0.0	0		0 - Normal
55	NO	NO	0.0	0		0 - Normal
56	NO	NO	0.0	0		0 - Normal
57	NO	NO	0.0	0		0 - Normal
58	NO	NO	0.0	0	•	0 - Normal
59	NO	NO	0.0	0	•	0 - Normal
60	NO	NO	0.0	0	•	0 - Normal
61	NO	NO	0.0	0	•	0 - Normal
62	NO	NO	0.0	0	•	0 - Normal
63	NO	NO	0.0	0	•	0 - Normal
64	NO	NO	0.0	0	•	0 - Normal

Detector Names

Det	33:	Detector	33	Det 49: Detector 4	19
Det	34:	Detector	34	Det 50: Detector 5	50
Det	35:	Detector	35	Det 51: Detector 5	51
Det	36:	Detector	36	Det 52: Detector 5	52
Det	37:	Detector	37	Det 53: Detector 5	53
Det	38:	Detector	38	Det 54: Detector 5	54
Det	39:	Detector	39	Det 55: Detector 5	55
Det	40:	Detector	40	Det 56: Detector 5	56
Det	41:	Detector	41	Det 57: Detector 5	57
Det	42:	Detector	42	Det 58: Detector 5	58
Det	43:	Detector	43	Det 59: Detector 5	59
Det	44:	Detector	44	Det 60: Detector 6	50
Det	45:	Detector	45	Det 61: Detector 6	51
Det	46:	Detector	46	Det 62: Detector 6	52
Det	47:	Detector	47	Det 63: Detector 6	53
Det	48:	Detector	48	Det 64: Detector 6	54

Detector Phase Assignment

						Pha	.se					
Det.	1	2	3	4	5	6	7	8	9	10	11	12
1				X								
1 2				X	•	•						
3				X	•	•						
4				X	•	•			•			
5 6	•	X			•	•			•			
6	•	X			•	•			•			
7	•	X			•	•			•			
8 9	•	X			•	•			•			
9	•	•		•	•	•		X	•			
10	•	•		•	•	•		X	•			
11				•	•	•		X	•			
12				•	•	•		X	•			
13		•	•	•	•	X		•	•		•	
14	•	•	•	•	•	X		•	•	•	•	
15	•	•	•	•	•	X		•	•	•	•	
16	•	•	•	•	•	X		•	•	•	•	
17					•	•			•			
18	•	•	•	•	•	•	•	•	•		•	•
19	•	•	•	•	•	•	•	•	•		•	•
20	•	•	•	•	•	•	•	•	•		•	•
21	•	•	•	•	•	•	•	•	•		•	•
22	•	•	•	•	•	•	•	•	•	•	•	•
23	•	•	•	•	•	•	•	•	•	•	•	•
24	•	•	•	•	•	•		•	•	•	•	•
25	•	•	•	•	•	•		•	•	•	•	•
26	•	•	•	•	•	•	•	•	•	•	•	•
27	•	•	•	•	•	•	•	•	•	•	•	•
28	•	•	•	•	•	•	•	•	•	•	•	•
29	•	•	•	•	•	•		•	•	•	•	•
30	•	•	•	•	•	•		•	•	•	•	•
31	•	•	•	•	•	•		•	•	•	•	•
32	•	•	•	•	•	•	•	•	•	•	•	•

Detector Cross Switching

						Pha	se					
Det.	1	2	3	4	5	6	7	8	9	10	11	12
1		•	•		•	•	•	•		•		
2		•	•		•	•	•	•		•		
2 3		•			•	•	•					
4		•			•	•	•					
5		•			•	•	•					
5 6		•	•	•	•	•	•	•	•	•	•	
7		•			•	•	•					
8		•			•	•	•					
9		•			•	•	•					
10		•	•		•	•	•					
11		•	•		•	•	•					
12		•	•		•	•	•					
13		•	•		•	•	•					
14		•	•		•	•	•					
15		•	•		•	•	•					
16		•	•		•	•	•					
17		•	•		•	•	•					
18		•			•	•	•					
19		•			•	•	•					
20		•			•	•	•					
21		•			•	•	•					
22		•			•	•	•					
23		•			•	•	•					
24		•			•	•	•					
25		•			•	•	•					
26		•			•	•	•					
27		•			•	•	•					
28		•			•	•	•					
29		•			•	•	•					
30		•					•					
31		•					•					
32		•	•	•	•	•	•	•		•	•	•

Detector Cross Switching

						Pha	se					
Det.	1	2	3	4	5	6	7	8	9	10	11	12
33	•											
34	•											
35	•					•		•				
36	•					•		•				
37	•			•		•			•			•
38				•		•			•		•	
39				•		•			•		•	
40	•	•	•	•	•	•	•	•	•	•	•	•
41	•	•	•	•	•	•	•	•	•	•	•	•
42	•	•	•	•	•	•	•	•	•	•	•	•
43						•		•	•	•	•	•
44						•		•	•	•	•	•
45						•		•	•	•	•	•
46	•	•	•	•	•	•	•	•	•	•	•	•
47	•	•	•	•	•	•	•	•	•	•	•	•
48	•	•	•	•	•	•	•	•	•	•	•	•
49		•		•	•	•	•	•	•	•	•	•
50	•					•		•	•	•	•	•
51	•			•		•		•	•	•	•	•
52	•	•	•	•	•	•	•	•	•	•	•	•
53	•	•	•	•	•	•	•	•	•	•	•	•
54	•	•	•	•	•	•	•	•	•	•	•	•
55	•	•	•	•	•	•	•	•	•	•	•	•
56	•	•	•	•	•	•	•	•	•	•	•	•
57	•	•	•	•	•	•	•	•	•	•	•	•
58	•	•	•	•	•	•	•	•	•	•	•	•
59	•	•	•	•	•	•	•	•	•	•	•	•
60	•	•	•	•	•	•	•	•	•	•	•	•
61	•	•	•	•	•	•	•	•	•	•	•	•
62	•	•	•	•	•	•	•	•	•	•	•	•
63	•	•	•	•	•	•	•	•	•	•	•	•
64	·	•	•	•	•	•	•	•	•	•	•	•

Bartholomew County SR 46 @ 325 W 2/3/2016 10:04

SDB1 = 3 & 11 SDB2 = 4 & 12 SDC1 = 5 & 13 SDC2 = 6 & 14 SDD1 = 7 & 15 SDD2 = 8 & 16

Diagnostic Plans/Fail Action

									Dete	ctor							
Pl	an	1	2	3	4	5	6	7	8	9	10	11	12	13	14	15	16
1	Diagnostic	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
	Scaling	1	1	1	1	1	1	1	1	1	1	1	1	1	1	1	1
2	Diagnostic	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
	Scaling	1	1	1	1	1	1	1	1	1	1	1	1	1	1	1	1
3	Diagnostic	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
	Scaling	1	1	1	1	1	1	1	1	1	1	1	1	1	1	1	1
4	Diagnostic	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
	Scaling	1	1	1	1	1	1	1	1	1	1	1	1	1	1	1	1
5	Diagnostic	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
	Scaling	1	1	1	1	1	1	1	1	1	1	1	1	1	1	1	1
6	Diagnostic	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
	Scaling	1	1	1	1	1	1	1	1	1	1	1	1	1	1	1	1
7	Diagnostic	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
	Scaling	1	1	1	1	1	1	1	1	1	1	1	1	1	1	1	1
8	Diagnostic	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
	Scaling	1	1	1	1	1	1	1	1	1	1	1	1	1	1	1	1
*F	ail Action	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
									Dete	ctor							
Pl	an	17	18	19	20	21	22		Dete 24		26	27	28	29	30	31	32
		17 0	18	19 0	20	21	22	23	Dete 24 0	ctor 25 0	26 0	27 0	28	29 0	30	31	32
Pl 1	Diagnostic							23	24	25							
	Diagnostic Scaling	0	0	0	0	0	0	23 0	24	25 0	0	0	0	0	0	0	0
1	Diagnostic Scaling Diagnostic	0 1	0 1	0 1	0 1	0 1	0 1	23 0 1	24 0 1	25 0 1	0 1	0 1	0 1	0 1	0 1	0 1	0 1
1	Diagnostic Scaling	0 1 0	0 1 0	0 1 0	0 1 0	0 1 0	0 1 0	23 0 1 0	24 0 1 0	25 0 1 0	0 1 0	0 1 0	0 1 0	0 1 0	0 1 0	0 1 0	0 1 0
1	Diagnostic Scaling Diagnostic Scaling	0 1 0 1	0 1 0 1	0 1 0 1	0 1 0 1	0 1 0 1	0 1 0 1	23 0 1 0	24 0 1 0	25 0 1 0 1	0 1 0 1	0 1 0 1	0 1 0 1	0 1 0 1	0 1 0 1	0 1 0 1	0 1 0 1
1	Diagnostic Scaling Diagnostic Scaling Diagnostic	0 1 0 1 0	0 1 0 1 0	0 1 0 1 0	0 1 0 1 0	0 1 0 1 0	0 1 0 1 0	23 0 1 0 1 0	24 0 1 0 1	25 0 1 0 1	0 1 0 1 0	0 1 0 1 0	0 1 0 1 0	0 1 0 1 0	0 1 0 1 0	0 1 0 1 0	0 1 0 1 0
1 2 3	Diagnostic Scaling Diagnostic Scaling Diagnostic Scaling	0 1 0 1 0	0 1 0 1 0	0 1 0 1 0	0 1 0 1 0	0 1 0 1 0	0 1 0 1 0	23 0 1 0 1 0	24 0 1 0 1 0	25 0 1 0 1 0	0 1 0 1 0	0 1 0 1 0	0 1 0 1 0	0 1 0 1 0	0 1 0 1 0	0 1 0 1 0	0 1 0 1 0
1 2 3	Diagnostic Scaling Diagnostic Scaling Diagnostic Scaling Diagnostic	0 1 0 1 0 1	0 1 0 1 0 1	0 1 0 1 0 1	0 1 0 1 0 1	0 1 0 1 0 1	0 1 0 1 0 1	23 0 1 0 1 0 1	24 0 1 0 1 0 1	25 0 1 0 1 0 1	0 1 0 1 0 1	0 1 0 1 0 1	0 1 0 1 0 1	0 1 0 1 0 1	0 1 0 1 0 1	0 1 0 1 0 1	0 1 0 1 0 1
1 2 3 4	Diagnostic Scaling Diagnostic Scaling Diagnostic Scaling Diagnostic Scaling	0 1 0 1 0 1 0	0 1 0 1 0 1 0	0 1 0 1 0 1 0	0 1 0 1 0 1 0	0 1 0 1 0 1 0	0 1 0 1 0 1	23 0 1 0 1 0 1 0	24 0 1 0 1 0 1 0	25 0 1 0 1 0 1 0	0 1 0 1 0 1	0 1 0 1 0 1 0	0 1 0 1 0 1 0	0 1 0 1 0 1 0	0 1 0 1 0 1 0	0 1 0 1 0 1 0	0 1 0 1 0 1
1 2 3 4	Diagnostic Scaling Diagnostic Scaling Diagnostic Scaling Diagnostic Scaling Diagnostic	0 1 0 1 0 1 0 1	0 1 0 1 0 1 0 1	0 1 0 1 0 1 0 1	0 1 0 1 0 1 0 1	0 1 0 1 0 1 0 1	0 1 0 1 0 1 0 1	23 0 1 0 1 0 1 0 1	24 0 1 0 1 0 1 0 1	25 0 1 0 1 0 1 0 1	0 1 0 1 0 1 0	0 1 0 1 0 1 0	0 1 0 1 0 1 0 1	0 1 0 1 0 1 0 1	0 1 0 1 0 1 0 1	0 1 0 1 0 1 0 1	0 1 0 1 0 1 0
1 2 3 4 5	Diagnostic Scaling Diagnostic Scaling Diagnostic Scaling Diagnostic Scaling Diagnostic Scaling Diagnostic Scaling	0 1 0 1 0 1 0 1	0 1 0 1 0 1 0 1	0 1 0 1 0 1 0 1	0 1 0 1 0 1 0 1	0 1 0 1 0 1 0 1	0 1 0 1 0 1 0 1	23 0 1 0 1 0 1 0 1	24 0 1 0 1 0 1 0 1	25 0 1 0 1 0 1 0 1	0 1 0 1 0 1 0 1	0 1 0 1 0 1 0 1	0 1 0 1 0 1 0 1	0 1 0 1 0 1 0 1	0 1 0 1 0 1 0 1	0 1 0 1 0 1 0 1	0 1 0 1 0 1 0 1
1 2 3 4 5	Diagnostic Scaling Diagnostic	0 1 0 1 0 1 0 1 0	0 1 0 1 0 1 0 1 0	0 1 0 1 0 1 0 1 0 1	0 1 0 1 0 1 0 1 0	0 1 0 1 0 1 0 1 0	0 1 0 1 0 1 0 1 0 1	23 0 1 0 1 0 1 0 1	24 0 1 0 1 0 1 0 1 0 1	25 0 1 0 1 0 1 0 1 0 1	0 1 0 1 0 1 0 1 0 1	0 1 0 1 0 1 0 1 0 1	0 1 0 1 0 1 0 1 0 1	0 1 0 1 0 1 0 1 0	0 1 0 1 0 1 0 1 0 1	0 1 0 1 0 1 0 1 0 1	0 1 0 1 0 1 0 1 0 1
1 2 3 4 5	Diagnostic Scaling Company Diagnostic Scaling Diagnostic Scaling Diagnostic Scaling	0 1 0 1 0 1 0 1 0 1 0 1	0 1 0 1 0 1 0 1 0 1 0	0 1 0 1 0 1 0 1 0 1	0 1 0 1 0 1 0 1 0 1	0 1 0 1 0 1 0 1 0 1	0 1 0 1 0 1 0 1 0 1	23 0 1 0 1 0 1 0 1 0 1	24 0 1 0 1 0 1 0 1 0 1	25 0 1 0 1 0 1 0 1 0 1	0 1 0 1 0 1 0 1 0 1 0	0 1 0 1 0 1 0 1 0 1 0	0 1 0 1 0 1 0 1 0 1	0 1 0 1 0 1 0 1 0 1	0 1 0 1 0 1 0 1 0 1	0 1 0 1 0 1 0 1 0 1	0 1 0 1 0 1 0 1 0 1 0 1
1 2 3 4 5	Diagnostic Scaling Diagnostic	0 1 0 1 0 1 0 1 0 1 0	0 1 0 1 0 1 0 1 0 1	0 1 0 1 0 1 0 1 0 1	0 1 0 1 0 1 0 1 0 1	0 1 0 1 0 1 0 1 0 1	0 1 0 1 0 1 0 1 0 1	23 0 1 0 1 0 1 0 1 0 1	24 0 1 0 1 0 1 0 1 0 1 0 1	25 0 1 0 1 0 1 0 1 0 1 0 1	0 1 0 1 0 1 0 1 0 1 0 1	0 1 0 1 0 1 0 1 0 1 0 1	0 1 0 1 0 1 0 1 0 1 0 1	0 1 0 1 0 1 0 1 0 1	0 1 0 1 0 1 0 1 0 1 0 1	0 1 0 1 0 1 0 1 0 1 0 1	0 1 0 1 0 1 0 1 0 1 0 1
1 2 3 4 5 6 7 8	Diagnostic Scaling Company Diagnostic Scaling Diagnostic Scaling Diagnostic Scaling	0 1 0 1 0 1 0 1 0 1 0 1	0 1 0 1 0 1 0 1 0 1 0	0 1 0 1 0 1 0 1 0 1 0	0 1 0 1 0 1 0 1 0 1 0	0 1 0 1 0 1 0 1 0 1 0	0 1 0 1 0 1 0 1 0 1 0	23 0 1 0 1 0 1 0 1 0 1	24 0 1 0 1 0 1 0 1 0 1 0 1	25 0 1 0 1 0 1 0 1 0 1 0 1	0 1 0 1 0 1 0 1 0 1 0	0 1 0 1 0 1 0 1 0 1 0	0 1 0 1 0 1 0 1 0 1 0	0 1 0 1 0 1 0 1 0 1 0	0 1 0 1 0 1 0 1 0 1 0	0 1 0 1 0 1 0 1 0 1 0	0 1 0 1 0 1 0 1 0 1 0 1

^{*}NOTE: 0 = No Action, 1 = Min Recall, 2 = Max Recall in Effect 3 = Detector Fail Max Tiime from By-Phase Timing Data

Diagnostic Plans/Fail Action

									Dete	ctor							
Pl	an	33	34	35	36	37	38	39	40	41	42	43	44	45	46	47	48
1	Diagnostic	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
	Scaling	1	1	1	1	1	1	1	1	1	1	1	1	1	1	1	1
2	Diagnostic	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
	Scaling	1	1	1	1	1	1	1	1	1	1	1	1	1	1	1	1
3	Diagnostic	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
	Scaling	1	1	1	1	1	1	1	1	1	1	1	1	1	1	1	1
4	Diagnostic	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
	Scaling	1	1	1	1	1	1	1	1	1	1	1	1	1	1	1	1
5	Diagnostic	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
	Scaling	1	1	1	1	1	1	1	1	1	1	1	1	1	1	1	1
6	Diagnostic	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
	Scaling	1	1	1	1	1	1	1	1	1	1	1	1	1	1	1	1
7	Diagnostic	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
	Scaling	1	1	1	1	1	1	1	1	1	1	1	1	1	1	1	1
8	Diagnostic	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
	Scaling	1	1	1	1	1	1	1	1	1	1	1	1	1	1	1	1
*F	ail Action	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
									Dete	ctor							
Pl	an	49	50	51	52	53	54			ctor 57	58	59	60	61	62	63	64
		49	50 0	51 0	52 0	53 0	54 0	55 0	Dete 56 0	57	58 0	59 0	60 0	61 0	62 0	63 0	64 0
Pl 1	Diagnostic							55	56			59 0 1	60 0 1				
	Diagnostic Scaling	0	0	0	0	0	0	55 0	56 0	57 0	0	0	0	0	0	0	0
1	Diagnostic Scaling Diagnostic	0 1	0 1	0 1	0 1	0 1	0 1	55 0 1	56 0 1	57 0 1	0 1	0 1	0 1	0 1	0 1	0 1	0
1	Diagnostic Scaling Diagnostic Scaling	0 1 0	0 1 0	0 1 0	0 1 0	0 1 0	0 1 0	55 0 1 0	56 0 1 0	57 0 1 0	0 1 0						
1	Diagnostic Scaling Diagnostic	0 1 0 1	0 1 0 1	0 1 0 1	0 1 0 1	0 1 0 1	0 1 0 1	55 0 1 0 1	56 0 1 0	57 0 1 0 1	0 1 0 1						
1	Diagnostic Scaling Diagnostic Scaling Diagnostic	0 1 0 1 0	0 1 0 1 0	0 1 0 1 0	0 1 0 1 0	0 1 0 1 0	0 1 0 1 0	55 0 1 0 1 0	56 0 1 0 1	57 0 1 0 1	0 1 0 1 0						
1 2 3	Diagnostic Scaling Diagnostic Scaling Diagnostic Scaling	0 1 0 1 0	0 1 0 1 0	0 1 0 1 0	0 1 0 1 0	0 1 0 1 0	0 1 0 1 0	55 0 1 0 1 0	56 0 1 0 1 0	57 0 1 0 1 0	0 1 0 1 0						
1 2 3	Diagnostic Scaling Diagnostic Scaling Diagnostic Scaling Diagnostic	0 1 0 1 0 1 0	0 1 0 1 0 1	0 1 0 1 0 1	0 1 0 1 0 1	0 1 0 1 0 1	0 1 0 1 0 1	55 0 1 0 1 0	56 0 1 0 1 0	57 0 1 0 1 0 1	0 1 0 1 0 1						
1 2 3 4	Diagnostic Scaling Diagnostic Scaling Diagnostic Scaling Diagnostic Scaling	0 1 0 1 0 1 0	0 1 0 1 0 1 0	0 1 0 1 0 1 0	0 1 0 1 0 1 0	0 1 0 1 0 1 0	0 1 0 1 0 1 0	55 0 1 0 1 0 1 0	56 0 1 0 1 0 1	57 0 1 0 1 0 1	0 1 0 1 0 1	0 1 0 1 0 1 0	0 1 0 1 0 1 0	0 1 0 1 0 1 0	0 1 0 1 0 1 0	0 1 0 1 0 1 0	0 1 0 1 0 1 0
1 2 3 4	Diagnostic Scaling Diagnostic Scaling Diagnostic Scaling Diagnostic Scaling Diagnostic	0 1 0 1 0 1 0 1	0 1 0 1 0 1 0 1	0 1 0 1 0 1 0 1	0 1 0 1 0 1 0 1	0 1 0 1 0 1 0 1	0 1 0 1 0 1 0 1	55 0 1 0 1 0 1 0	56 0 1 0 1 0 1 0	57 0 1 0 1 0 1 0	0 1 0 1 0 1 0	0 1 0 1 0 1 0 1	0 1 0 1 0 1 0 1	0 1 0 1 0 1 0 1	0 1 0 1 0 1 0	0 1 0 1 0 1 0 1	0 1 0 1 0 1 0
1 2 3 4 5	Diagnostic Scaling Diagnostic Scaling Diagnostic Scaling Diagnostic Scaling Diagnostic Scaling	0 1 0 1 0 1 0 1	0 1 0 1 0 1 0 1	0 1 0 1 0 1 0 1	0 1 0 1 0 1 0 1	0 1 0 1 0 1 0 1	0 1 0 1 0 1 0 1	55 0 1 0 1 0 1 0 1	56 0 1 0 1 0 1 0 1	57 0 1 0 1 0 1 0 1	0 1 0 1 0 1 0 1						
1 2 3 4 5	Diagnostic Scaling Diagnostic Scaling Diagnostic Scaling Diagnostic Scaling Diagnostic Scaling Diagnostic Scaling	0 1 0 1 0 1 0 1 0	0 1 0 1 0 1 0 1 0	0 1 0 1 0 1 0 1 0	0 1 0 1 0 1 0 1 0	0 1 0 1 0 1 0 1 0	0 1 0 1 0 1 0 1 0	55 0 1 0 1 0 1 0 1	56 0 1 0 1 0 1 0 1	57 0 1 0 1 0 1 0 1	0 1 0 1 0 1 0 1 0	0 1 0 1 0 1 0 1 0	0 1 0 1 0 1 0 1 0	0 1 0 1 0 1 0 1 0	0 1 0 1 0 1 0 1 0	0 1 0 1 0 1 0 1 0	0 1 0 1 0 1 0 1 0 1
1 2 3 4 5	Diagnostic Scaling Diagnostic Scaling Diagnostic Scaling Diagnostic Scaling Diagnostic Scaling Diagnostic Scaling Company Diagnostic Scaling Diagnostic Scaling Diagnostic Scaling	0 1 0 1 0 1 0 1 0 1	0 1 0 1 0 1 0 1 0 1	0 1 0 1 0 1 0 1 0 1	0 1 0 1 0 1 0 1 0 1	0 1 0 1 0 1 0 1 0 1	0 1 0 1 0 1 0 1 0 1	55 0 1 0 1 0 1 0 1 0	56 0 1 0 1 0 1 0 1 0 1	57 0 1 0 1 0 1 0 1 0 1	0 1 0 1 0 1 0 1 0 1	0 1 0 1 0 1 0 1 0 1	0 1 0 1 0 1 0 1 0 1	0 1 0 1 0 1 0 1 0 1	0 1 0 1 0 1 0 1 0	0 1 0 1 0 1 0 1 0 1	0 1 0 1 0 1 0 1 0 1
1 2 3 4 5	Diagnostic Scaling Diagnostic	0 1 0 1 0 1 0 1 0 1	0 1 0 1 0 1 0 1 0 1	0 1 0 1 0 1 0 1 0 1	0 1 0 1 0 1 0 1 0 1	0 1 0 1 0 1 0 1 0 1	0 1 0 1 0 1 0 1 0 1	55 0 1 0 1 0 1 0 1 0 1	56 0 1 0 1 0 1 0 1 0 1	57 0 1 0 1 0 1 0 1 0 1	0 1 0 1 0 1 0 1 0 1						
1 2 3 4 5 6 7 8	Diagnostic Scaling Company Diagnostic Scaling Diagnostic Scaling Diagnostic Scaling	0 1 0 1 0 1 0 1 0 1 0 1	0 1 0 1 0 1 0 1 0 1 0	0 1 0 1 0 1 0 1 0 1 0	0 1 0 1 0 1 0 1 0 1 0	0 1 0 1 0 1 0 1 0 1 0	0 1 0 1 0 1 0 1 0 1 0	55 0 1 0 1 0 1 0 1 0 1	56 0 1 0 1 0 1 0 1 0 1 0 1	57 0 1 0 1 0 1 0 1 0 1 0 1	0 1 0 1 0 1 0 1 0 1 0	0 1 0 1 0 1 0 1 0 1 0	0 1 0 1 0 1 0 1 0 1 0	0 1 0 1 0 1 0 1 0 1 0	0 1 0 1 0 1 0 1 0 1 0	0 1 0 1 0 1 0 1 0 1 0	0 1 0 1 0 1 0 1 0 1 0 1

^{*}NOTE: 0 = No Action, 1 = Min Recall, 2 = Max Recall in Effect 3 = Detector Fail Max Tiime from By-Phase Timing Data

Ped Diagnostic Plans

Plan		1	2	3	4	5	6	7	8	9	10	11	12
1	Diagnostic	0	0	0	0	0	0	0	0	0	0	0	0
	Scaling	1	1	1	1	1	1	1	1	1	1	1	1
2	Diagnostic	0	0	0	0	0	0	0	0	0	0	0	0
	Scaling	1	1	1	1	1	1	1	1	1	1	1	1
3	Diagnostic	0	0	0	0	0	0	0	0	0	0	0	0
	Scaling	1	1	1	1	1	1	1	1	1	1	1	1
4	Diagnostic	0	0	0	0	0	0	0	0	0	0	0	0
	Scaling	1	1	1	1	1	1	1	1	1	1	1	1
5	Diagnostic	0	0	0	0	0	0	0	0	0	0	0	0
	Scaling	1	1	1	1	1	1	1	1	1	1	1	1
6	Diagnostic	0	0	0	0	0	0	0	0	0	0	0	0
	Scaling	1	1	1	1	1	1	1	1	1	1	1	1
7	Diagnostic	0	0	0	0	0	0	0	0	0	0	0	0
	Scaling	1	1	1	1	1	1	1	1	1	1	1	1
8	Diagnostic	0	0	0	0	0	0	0	0	0	0	0	0
	Scaling	1	1	1	1	1	1	1	1	1	1	1	1

Detector Diagnostic Intervals

Diagnostic Number	*No-Activity Diagnostic Interval	*Max Presence Diagnostic Interval	Erratic Counts
1	0	0	0
2	0	0	0
3	0	0	0
4	0	0	0
5	0	0	0
6	0	0	0
7	0	0	0
8	0	0	0
9	0	0	0
10	0	0	0
11	0	0	0
12	0	0	0
13	0	0	0
14	0	0	0
15	0	0	0
16	0	0	0
17	0	0	0
18	0	0	0
19	0	0	0
20	0	0	0
21	0	0	0
22	0	0	0
23	0	0	0
24	0	0	0
25	0	0	0
26	0	0	0
27	0	0	0
28	0	0	0
29	0	0	0
30	0	0	0
31	0	0	0
32	0	0	0

^{*}NOTE: Scaling is specified in each detector diagnostic plan.

Speed Detectors

			Local	Spee	d Det	ector	•	
One Detector Speed:	1	2	3	4	5	6	7	8
Local Detector Number	0	0	0	0	0	0	0	0
Vehicle Length	0	0	0	0	0	0	0	0
Loop Length	0	0	0	0	0	0	0	0
Two Detector Speed:								
Local Detector Number	0	0	0	0	0	0	0	0
Speed Trap Length	0	0	0	0	0	0	0	0
One Debenhau Guarda	0	1.0	Local	_				1.6
One Detector Speed:	9	10	11					16
Local Detector Number	0	0	0	0	0	0	_	Ü
Vehicle Length	0	0	0	0	0	0	0	0
Toon Tongth								
Loop Length	0	0	0	0	0	0	0	0
Two Detector Speed:	0	0	0	0	0	0	0	0
	0	0	0	0	0	0	0	0

Units. Inches

NOTE: Speed Detector 1 = STA, Speed Detector 2 = STB

Manual Enable		Pa	tter	'n.				0					
Split Units Interconnect Format Transition Resync Count	t . PLAN SMOOTH			Int	erco		So	urce	. Pe		ent		
Actuated Coord Phase Inhibit Max Timing Floating Force Off		. Ma:	x 2	Sel	ect								
							P.	hase					
Split Demand: Call	Time Cyc	Count	1	2	3	4 5	6	7	8 9	10	11 1	2	
Demand 1	_	0											
Demand 2	0	0											
							Ph	ase					
		1	2	3	4	5	6	7	8	9	10	11	12
Auto Permissive Min	n Green .	0	0			0	0	0	0	0	0	0	0
		A	В	С	D	E	F						
Free Alternate Sequ	uence		-										
1 = 1 = 1 = 1 3 2 1 4 0 5 0 4		•	•	,	•	,	•						

Bartholomew County SR 46 @ 325 W 2/3/2016 10:04

Coordinator Manual Command and Options

Coordination Patterns

Preemptors		
Preemptor 1 Active		
Green		Yellow Red
Minimum 0 Track Clear 0		0.0 0.0 0.0
Hold		0.0 0.0
Phase/Overlap 1	2	3 4 5 6 7 8 9 10 11 12/ A B C D
Terminate Overlap		
Track Clearance Phase Hold Phases		• • • • • • • • • •
Exit Phases		
Exit Calls on Phase		
		es Green
Preemptor 2		Det Lock Ped Dark
		Yel-Red To Grn Ped Active
Outputs Only During Hold		
Terminate Overlap ASAP		
Don't Override Flash		Duration Time 0
Flash During Hold		Delay Time 0
No CVM in Flash		Inhibit Time 0
Fast Flash Grn on Hold Phase.		Min Ped Clear 0
Enable Max Time		Max Time 0
		Exit Max 0
		Min Hold Time 0
		Hold Delay Time 0
Green		Yellow Red
Minimum 0		0.0 0.0
Track Clear 0		0.0 0.0
Hold		0.0 0.0
Phana / O 1	2	2 4 5 6 7 0 0 10 11 10 / 7 7 7
_		3 4 5 6 7 8 9 10 11 12/ A B C D
Terminate Overlap Track Clearance Phase	•	
Hold Phases	•	
Exit Phases	•	
Exit Calls on Phase		
Out of Flash Color for Exit Ph Linked Preemptor 0		es Green

Preemptors															
Preemptor 3 Active		Det Yel Fla Ter Dur	Lo -Resh mir at: ay ib: Pe T: t N	ock ed Al nat ion Ti it ed ime Max	To (l O' e Pl Tin me Tim Cle . Tin	Grn utpo hase me. e . ar. e .	uts es.		. II	Ped Ped Zero	Dar Act Pe Clr	ive d C	: . !lr	 Tim	e.
Green		Yel					ed								
Minimum 0 Track Clear 0		_	.0			0									
Track Clear 0 Hold			.0				.0								
	_				_			^	1.0		10.	_	_	~	_
Phase/Overlap 1 Terminate Overlap			4					9	10	11	12/	A	В	С	D
Track Clearance Phase					•				•		•	•	•	•	•
Hold Phases									•						
Exit Phases Exit Calls on Phase	•	•				•			•	•	•				
Out of Flash Color for Exit Ph Linked Preemptor 0 Preemptor 4 Active	 · ·	Det Yel Fla	Lo -Re sh mir at: ay ib: Pe T: Ho d I	ock ed Al nat ion Ti ted Max old	 To (l O e P Tin me Tim Cle		uts		. II	Ped Ped Zero	Act	ive d C	: . !lr	 Tim	e.
Minimum 0		0	.0	,,		0	. 0								
Track Clear 0			.0				. 0								
Hold		U	. 0			U	. 0								
Phase/Overlap 1			4	5	6	7	8	9	10	11	12/	A	В	С	D
Terminate Overlap				٠	٠	•	•	•	•	•	•	•	•	•	•
Track Clearance Phase Hold Phases		•	•	•	•	•	•	•	•	•	•				
Exit Phases		•							•		•				
Exit Calls on Phase															
Out of Flash Color for Exit Ph Linked Preemptor 0	as	es .	•	•	. G:	reei	n 			-					

Preemptors														
Preemptor 5 Active	• • •	Det L Yel-R Flash Termi: Durat Delay Inhib Min P Max T Exit I	ock ed Al nat ion Ti ed ime Max	To l O e P Ti me Tim Cle	Grn utp hase me	uts es		. II	Ped Ped Zero	Dar Act	ive d C	: . !lr	 Tim	e.
Green Minimum 0 Track Clear 0 Hold		Yello 0.0 0.0 0.0	W		R 0 0	ed .0	•	0						
Hold Phases	• • •	· · · · · · · · · · · · · · · · · · ·				· · ·	•							D .
Out of Flash Color for Exit Pha Linked Preemptor 0	ase	es	•	. G	ree	n								
Preemptor 6 Active	· · · ·	Det L Yel-R Flash Termi	ock ed Al nat ion Ti ed ime Max	To l O e P Ti me Tim Cle	Grn utp hase me. e . ar. me.	uts es		. I	Ped Ped Zero	Act De	ive d C	!lr	 Tim	ie.
Green Minimum 0 Track Clear 0 Hold		Yello 0.0 0.0 0.0			0 0	ed .0 .0								
Hold Phases	•	· · · · · · · · · · · · · · · · · · ·						10	11	12/	A .	В .	C .	D .
Linked Preemptor 0									_					

Bus Preemptors

	Bus P	reempto	or			
1	2	3	4			
Preemptor Active	•	•				
Detector Lock	•	•				
Maximum Time 0	0	0	0			
Reservice Time 0	0	0	0			
Delay Time 0	0	0	0			
Inhibit Time 0	0	0	0			
Entrance Green 0	0	0	0			
Entrance Ped Clearance 0	0	0	0			
Entrance Yellow 0.0	0.0	0.0	0.0			
Entrance Red 0.0	0.0	0.0	0.0			
Minimum Hold Time 0	0	0	0			
	Hold	Phases				
1 2 3 4	5 6	7	8 9	10	11	12
Preemptor 1					•	
Preemptor 2		•		•		
Preemptor 3		•				
Preemptor 4						

Manual NIC Program Step 0
Manual TOD Program Step 0
NIC Resync Time 0000
Sync Reference is Reference Time
Week 1 Begins on 1st Sunday NO If NO, then week containing Jan. 1
Disable Daylight Savings Time NO
Daylight Savings Begins Last Sunday in March NO If NO, then Second Sunday as per 2007 DST Law

NIC/TOD Clock/Calendar

TOD Weekly/Yearly

						Wee	kly	Prog	ram	Numb	ers							
			1	2	3		4	5	6	7		8	9	10				
Sunda	у.		1	1	1		1	1	1	1		1	1	1	. P	rogr	am N	· .
Monda			1	1	1		1	1	1	1		1	1	1	P	rogr	am N	o.
Tuesd	ay.		1	1	1		1	1	1	1		1	1	1	P	rogr	am N	Ο.
Wedne	sday		1	1	1		1	1	1	1		1	1	1	P	rogr	am N	o.
Thurs	day		1	1	1		1	1	1	1		1	1	1	P	rogr	am N	o.
Frida	у.		1	1	1		1	1	1	1		1	1	1	P	rogr	am N	· .
Satur			1	1	1		1	1	1	1		1	1	1	P	rogr	am N	· .
								Wee	k of	Yea	r							
	1	2	3	4	5	6	7	8	9	10	11	12	13	14	15	16	17	18
Prog	1	1	1	1	1	1	1	1	1	1	1	1	1	1	1	1	1	1
	19	20	21	22	23	24	25	26	27	28	29	30	31	32	33	34	35	36
Prog	1	1	1	1	1	1	1	1	1	1	1	1	1	1	1	1	1	1
	37	38	39	40	41	42	43	44	45	46	47	48	49	50	51	52	53	
Prog	1	1	1	1	1	1	1	1	1	1	1	1	1	1	1	1	1	

Holiday Programs

Holiday	Type	Month	Day of Week/ Day of Month	Week of Year/ Year	Program
1	Fixed	0	0	0	0
2	Fixed	0	0	0	0
3	Fixed	0	0	0	0
4	Fixed	0	0	0	0
5	Fixed	0	0	0	0
6	Fixed	0	0	0	0
7	Fixed	0	0	0	0
8	Fixed	0	0	0	0
9	Fixed	0	0	0	0
10	Fixed	0	0	0	0
11	Fixed	0	0	0	0
12	Fixed	0	0	0	0
13	Fixed	0	0	0	0
14	Fixed	0	0	0	0
15	Fixed	0	0	0	0
16	Fixed	0	0	0	0
17	Fixed	0	0	0	0
18	Fixed	0	0	0	0
19	Fixed	0	0	0	0
20	Fixed	0	0	0	0
21	Fixed	0	0	0	0
22	Fixed	0	0	0	0
23	Fixed	0	0	0	0
24	Fixed	0	0	0	0
25	Fixed	0	0	0	0
26	Fixed	0	0	0	0
27	Fixed	0	0	0	0
28	Fixed	0	0	0	0
29	Fixed	0	0	0	0
30	Fixed	0	0	0	0
31	Fixed	0	0	0	0
32	Fixed	0	0	0	0
33	Fixed	0	0	0	0
34	Fixed	0	0	0	0
35	Fixed	0	0	0	0
36	Fixed	0	0	0	0

NIC Program Steps

Step Program Step Begins Pattern Override